

THIS AGREEMENT made in triplicate on the 1st day of October, 2018.

B E T W E E N:

THE CORPORATION OF THE MUNICIPALITY OF SOUTH HURON

(Hereinafter referred to as the "Municipality")

OF THE FIRST PART

– And –

HAMATHER MOTOR PRODUCTS

(Hereinafter referred to as the "Owner")

OF THE SECOND PART

WHEREAS the Owner is entering into this agreement with the Municipality dealing with the facilities, works and matters hereinafter mentioned and the provision and maintenance thereof by the Owner and any and all subsequent owners to the satisfaction of and at no expense to the Municipality, as a condition to the approval pursuant to Section 41 of the Planning Act, as amended, of site plans and drawings for a development (hereinafter called the "development") on the lands and premises of the Owner more particularly described in the Schedule "A" attached hereto in the Municipality of South Huron, in the County of Huron (the "property").

NOW THEREFORE WITNESSETH THAT in consideration of the covenants and provisions herein and for other good and valuable consideration now paid by the Municipality to the Owner (the receipt and sufficiency of which the Owner hereby acknowledges), the Municipality and the Owner covenant, agree and provide with each other that the Owner shall do and perform, at no expense to the Municipality (unless otherwise expressly provided herein), the following matters and things:

1. DRAWINGS AND CONSTRUCTION OF EXTERNAL WORKS:

The Owner shall submit to and have approved by the Municipality detailed design drawings of external road, sewer and other improvements, together with associated internal works. The Owner shall construct, prior to occupancy of any building (unless otherwise approved by the Municipality), at the Owner's expense and to the satisfaction of the Municipality, the following works:

- (a) water service;
- (b) fire protection;
- (c) sanitary sewer service;
- (d) roadways;
- (e) signage;
- (f) stormwater management;
- (g) lighting

2. STORMWATER MANAGEMENT:

The Owner shall undertake all work required to implement the Drainage Report prepared by AGM on July 31, 2018 and approved by the Municipal Engineer. The approved grading and servicing is incorporated into the attached Schedule "C".

The Owner agrees to maintain the property in such a manner that ensures compliance with the approved Stormwater Management Plan.

3. PARKING:

- (a) Parking Dimensions are sufficient and meet minimum zone provisions for size as proposed on the Site Plan incorporated as Schedule "B".
- (b)

4. LIGHTING FACILITIES:

All lighting of the site shall be oriented and its intensity controlled so as to prevent glare on adjacent roadways and adjacent properties to the satisfaction of the Municipality. Provide confirmation that lighting has been completed in accordance with the approved Site Plan, which forms Schedule "B" herein.

5. LANDSCAPING

The Owner shall landscape the site and thereafter maintain the same in general conformity with the approved Site Plan attached hereto as Schedule "B", to the satisfaction of the Municipality of South Huron.

The Owner shall provide a landscape plan to the satisfaction of the Municipality of South Huron prior to implementation.

6. FIRE ROUTE DESIGNATION:

The Owner shall identify the fire route. Such fire route shall be clearly marked showing street allowances and vehicular accesses for the approval of the Fire Chief. Signs specifying that parking is prohibited in the designated fire route shall be displayed.

7. 'AS CONSTRUCTED' PREMISES:

The Owner shall provide for the Municipality's records 'as constructed' drawings to the satisfaction of the Municipality for municipal services installed by the Owner which may, in the future, be assumed by the Municipality. These drawings shall be submitted in a satisfactory form prior to the release of any performance bond or security required by this agreement. The development shall be completed in accordance with Schedule "D" herein.

8. INSPECTION AND COMPLETION OF WORKS:

Where the Owner is required to construct certain works to be assumed by the Municipality or carry out work within a public highway, walkway or easement, the Owner shall have his Professional Engineer provide a qualified inspector acceptable to the Municipality to carry out on-site inspection of the works. Upon completion of the work and prior to requesting the Municipality to assume the works, the Owner shall supply to the Municipality, in a form acceptable, a certificate of the Owner's Professional Engineer substantially in the following form:

CERTIFICATE OF COMPLETION OF WORKS

TO: The Corporation of the Municipality of South Huron

For good and valuable consideration now paid by the Corporation of the Municipality of South Huron (hereinafter called the "MUNICIPALITY"), the receipt and sufficiency of which I/we hereby acknowledge, I/we hereby certify that the municipal services constructed pursuant to the Development Agreement between the Municipality and (Owner's Name) registered as No. _____ relating to municipal number Lot/Block No. _____ Plan No. _____ have been

- (a) inspected during construction in accordance with standard engineering practice; and
- (b) constructed in accordance with the plans and specifications approved by the Municipality.

Delivered under my/our hand and professional seal at South Huron, Ontario this _____ day of _____, 20_____.

Registered Professional Engineer

The Owner acknowledges and agrees that the form of the Certificate of Completion of Works required under this paragraph may vary depending on the development's requirements.

9. SUBSURFACE DRAINAGE:

The Owner shall notify the Municipality, in writing, in the event that any existing sewer or drain is encountered during the progress of construction. The Owner shall have its Engineer investigate the matter and shall comply with the recommendations of the Owner's Engineer, as approved by the Municipality, with respect to the sewer or drain encountered. Such recommendations may include connecting the existing sewer to a new sewer being constructed or into another existing sewer, at no expense to the Municipality. The Owner shall also ensure that there is no interruption of any subsurface drainage flow because of construction on the site which would have an adverse effect on neighbouring properties. Should such an interruption occur, the Owner shall carry out any necessary remedial work to correct the problem as requested by the Municipality and to the satisfaction of the Municipality at no expense to the Municipality.

10. ABANDONED PRIVATE DRAIN CONNECTIONS:

The Owner acknowledges that any abandoned existing private drain connections shown on the site plans or encountered during construction are to be excavated at the street line and sealed to the satisfaction of the Municipality.

11. EXISTING PRIVATE DRAIN CONNECTIONS:

The Owner acknowledges that any existing private drain connections which are proposed for re-use are to be excavated at the street line and inspected and approved by the Municipality for such re-use.

12. UNDERTAKING OF CONSTRUCTION:

If no building permit is issued for the development within two (2) years of the date of the approval of the site plans and drawings pursuant to Section 41 of the Planning Act, (Ontario), as amended, or if a building permit is issued but, in the opinion of the Chief Building Official, the Owner does not seriously commence construction of the development within two (2) years from the date of the approval of the site plans and drawings pursuant to Section 41 of the Planning Act (Ontario), as amended, or if any building permit issued for this development is revoked at any time, the Municipality in its sole discretion may revoke its approval of the plans and drawings and may terminate the agreement by giving notice in writing and by registering a notice that the approval is revoked and the agreement is terminated.

13. WORK ACCORDING TO PLANS:

As the Owner has entered into this agreement as a condition precedent to the approval by the Municipality of site plans and drawings dealing with the facilities, works and matters mentioned herein, the Owner shall submit from time to time one or more plans and drawings as may be required pertaining to any of these facilities, works and matters including but not restricted to any plans or drawings specifically mentioned herein. Such plans and drawings as and when approved by the Municipality, whether before or after the date upon which this agreement is entered into, shall be treated as forming part of this agreement in the same manner and to the same extent as if such plans and drawings had been approved and actually attached to this agreement at the time that it is entered into. In all matters not herein provided for, the Owner shall develop his land and shall use the same in accordance with the applicable Zoning By-Law of the Municipality, as amended. The provisions of this agreement and any approved site plan or drawing pertaining to a facility, work or matter shall be construed and applied as complementary to each other but in the event of any conflict, the plan or drawing receiving the last approval shall govern. Without restricting the generality of this clause, the Owner shall develop his lands and shall construct works and maintain them in perpetuity in accordance with the approved Site Servicing Plan which is attached as Schedule "B".

14. WORK AT OWNER'S RISK:

All incidental matters including but not restricted to the removal and planting of trees; cutting, replacing and installing approaches; relocating utilities, pipes, poles, valves and equipment; resetting drains and manholes; and all other things required by this agreement or by the Municipality shall be carried out by the Owner at his own risk and expense. All work must be completed to the satisfaction of the Municipality and to the satisfaction of the owner of such utilities.

15. COMPLETION OF WORK:

All work required under this agreement, including but not restricted to asphalt surfacing, fencing, establishment of landscaping and as constructed drawings, completion of services and any other work set out herein, shall be completed or delivered, as the case may be, within a period of nine (9) months from the date of substantial completion of construction of the development as determined by the Chief Building Official. All such work shall be performed to the satisfaction of the Chief Building Official of the Municipality.

16. SECURITIES:

In order to ensure due performance of all work required under this agreement and to protect the Municipality in respect of its liability for holdback of costs under Section 17 of the Construction Lien Act (Ontario), as amended, for any work on municipal property, the Owner shall deposit with the Municipality prior to the issuance of a building permit, an irrevocable Letter of Credit from a chartered bank, issued in form and content satisfactory to the Municipality's Solicitor, in the amount of One Hundred Percent (100%) of the total securities as set out in Schedule "F".

All Letters of Credit shall be for a minimum guaranteed period of one (1) year or such longer time as the Municipality may decide. All Letters of Credit shall contain the following clause: "It is a condition of the Letter of Credit that it shall be deemed to be automatically extended without amendment from year to year from the present or any future expiration date thereof, unless at least thirty (30) days prior to the present or any future expiration date, we notify you in writing by registered mail that we elect not to consider this Letter of Credit to be renewable for any additional period." Unless each and every Letter of Credit is renewed as noted above, the Municipality shall have the absolute right to refuse to issue building permits and to prohibit occupancy, whether partially or fully completed, from the said date thirty (30) days prior to the expiration of that Letter of Credit.

16.1. SECURITY RELEASE

General securities outlined in Schedule "F" will be released upon the completion of all works, to the satisfaction of the Municipality of South Huron.

17. DEVELOPMENT CHARGES:

The Owner shall pay all development charges applicable to the development in accordance with the By-laws of the Municipality of South Huron.

18. MUNICIPALITY'S RIGHT TO ENTER:

The Municipality or any of its officers, servants or agents may, from time to time, at all reasonable times and upon producing proper identification, enter upon the Owner's lands and premises for the purposes of inspecting the facilities, works and matters to be provided and maintained under this agreement and for the purpose of providing or maintaining at the Owner's expense any facility, work or matter in default of the Owner providing or maintaining the same where such default has continued for fifteen (15) days or more. The Municipality, its officers, servants and agents shall not be liable to the Owner or any occupant of the lands and premises for any losses or damages of any kind whatsoever arising in any way from entry for such purposes. In the event of an emergency, the Municipality's right to enter under this provision shall not be limited to situations in which the default of the Owner has continued for more than fifteen (15) days.

19. ROAD ALLOWANCE INDEMNITY:

Except as otherwise expressly provided in this agreement, the right of the Owner to use and occupy any untravelled portions of road allowances shall, at all times, be at the will of the Municipality and the construction and maintenance of any and all curbs, pavements, plantings and other improvements or works thereon shall at all times be at the risk and expense of the Owner. The Owner shall indemnify and save harmless the Municipality and any of its officers, employees or servants from and against all actions, suits, claims, damages, demands, costs, including reasonable legal fees and disbursements, liabilities and any other claims which may be brought against or made upon the Municipality or any of its officers, employees or servants in consequence of the use and occupation of untraveled portions of road allowances by the Owner or the construction, maintenance or existence of curbs, pavements, plantings or other improvements of the Owner thereon. Any amounts owed by the Owner to the Municipality under this indemnity shall constitute a lien and charge upon the lands of the Owner and shall be collectible in like manner as municipal taxes. Without limiting the foregoing agreement to indemnify, the Municipality may, in case any such action, suit, claim or demand is brought or made against the Municipality or any of its officers, employees or servants, settle any such action, suit, claim or demand on such terms as the Municipality shall see fit, and the Owner shall thereupon forthwith pay to the Municipality the sum or sums so paid, together with such sum as shall represent the reasonable costs of the Municipality and its solicitor in defending or settling any such action, suit, claim or demand.

20. INSURANCE:

Prior to the issuance of any building permit and any commencement of work for the development, the Owner shall supply the Municipality with a certified copy of a comprehensive general liability insurance policy with limits in an amount and in a form acceptable to the Municipality. The minimum limits of such policies shall be \$5,000,000 all inclusive, but the Municipality shall have the right to set higher amounts. Such policy or policies shall be issued in the joint names of the Owner and the Municipality. The said insurance policy shall indemnify the Municipality from any loss arising from any claims for damages, injury or otherwise in connection with the work done by or on behalf of the Owner. Such insurance policy shall provide coverage for a period of at least one (1) year and shall continue until all the work required by the Owner under this Agreement is completed and, where applicable, assumed by the Municipality. The said insurance policy must also include a provision confirming that the insurance policy shall not be cancelled or materially amended without providing the Municipality with thirty (30) days' written notice of the insurer's intention to do so. The issuance of such a policy of insurance shall not be construed as relieving the Owner from responsibility for other or larger claims, if any, for which he may be held responsible.

21. GENERAL INDEMNITY:

The Owner shall indemnify and save harmless the Municipality and any of its officers, employees or servants from and against all actions, suits, claims, damages, demands, costs, including reasonable legal fees and disbursements, liabilities and any other claims which may be brought against or made upon the Municipality or any of its officers, employees or servants sustained or incurred by the Municipality or any of its officers, employees or servants as a result of the Municipality entering into this agreement with the Owner. Any amounts owed by the Owner to the Municipality under this indemnity shall constitute a lien and charge upon the lands of the Owner and shall be collectible in like manner as municipal taxes. Without limiting the foregoing agreement to indemnify, the Municipality may, in case any such action, suit, claim or demand is brought or made against the Municipality or any of its officers, employees or servants, settle any such action, suit, claim or demand on such terms as the Municipality shall see fit, and the Owner shall thereupon forthwith pay to the Municipality the sum or sums so paid, together with such sum as shall represent the reasonable costs of the Municipality and its solicitor in defending or settling any such action, suit, claim or demand.

22. BY-LAWS:

Notwithstanding any of the provisions of this agreement, the Owner shall be subject to all By-Laws of the Municipality. In the event of conflict between the provisions of this agreement and the provisions of any By-Law of the Municipality, the provisions of the By-Law prevail.

23. SUBSEQUENT OWNERS BOUND:

Subject to the provisions of the Registry Act and the Land Titles Act, the covenants, agreements, conditions and understandings therein contained on the part of the Owner shall be conditions running with the land described in Schedule "A" hereto and shall be binding upon the Owner and their heirs, estate trustees, administrators, successors and assigns, as the case may be, and subsequent owners and occupiers of the said lands from time to time (and "Owner", wherever used in this agreement, is intended and shall be construed to include such subsequent owners and occupiers).

24. SEPARATE COVENANTS:

All of the provisions of this agreement are and shall be construed and interpreted as covenants and agreements as though the words importing such covenants and agreements were used in each separate clause hereof. Should any covenant or provision of this agreement be adjudged unlawful or unenforceable, such covenant or provision shall be considered separate, distinct and severable from this agreement and the covenants and provisions of this agreement shall not be affected and shall remain fully enforceable.

25. ENFORCING PERFORMANCE OF REQUIREMENTS:

In addition to any remedy authorized or permitted by this agreement or by law, the Municipality, upon giving fifteen (15) days notice or forthwith in cases of emergency, may, in default of any matter or thing required to be done by the Owner under this agreement, do such matter or thing at the expense of the Owner and if the Municipality has incurred any expense, it may recover the expense by action, by performance bond or other security or by adding the said expenses to the tax roll and recovering same in like manner as municipal taxes. No proceeding by the Municipality under this clause and no waiver under any provision of this agreement shall prejudice the rights of the Municipality in respect of any subsequent default or any matter or thing required to be done by the Owner under this agreement. The rights of the Municipality may be enforced by any remedy authorized or permitted by the Agreement or By-Law and no such remedy shall be exclusive or dependent on any other remedy.

26. NUMBER AND GENDER:

Words importing the singular only shall include the plural; words importing the masculine only shall include the female and words importing a person shall include corporations.

27. NOTICES:

Any notice required or permitted to be given hereunder shall be in writing and shall be effectively given if delivered personally or sent by registered mail in the case of notice to the Municipality as follows:

Municipality of South Huron
P.O. Box 759
322 Main Street South
Exeter ON, N0M 1S6

And in the case of notice to the Owners, as follows:

Mr. Tim Hamather
70704 London Road
Exeter, ON N0M 1S1

Any notice so given shall be deemed conclusively to have been given and received when so personally delivered or on the third (3rd) business day following the sending thereof by registered mail.

28. REGISTRATION:

The Owner agrees that this document shall be registered against the title to the lands affected by it and that such registration shall be done by the Municipality. The cost of such registration and associated legal fees shall be the responsibility of the Owner.

The Owner further agrees that this agreement shall have priority over all mortgages that are registered against the property and the Owner hereby undertakes to deliver an agreement postponing those mortgages to this agreement and to register the same on title.

29. COSTS:

Any costs incurred by the Municipality for the review, implementation and administration of this agreement (including engineering, administrative costs and legal fees) shall be borne by the Owner.

IN WITNESS WHEREOF the Municipality and the Owner hereto have hereunto affixed their Corporate Seals duly attested by the hands of their proper officers in that behalf, the day and year first written above.

THE CORPORATION OF THE MUNICIPALITY OF
SOUTH HURON

Per: Maureen Cole, Mayor

Per: Rebekah Msuya-Collison, Clerk

We have authority to bind the Corporation.

SIGNED, SEALED AND DELIVERED
In the presence of

Hamather Motor Products

Per: Tim Hamather, Duly Authorized Officer

I have the authority to bind the Corporation.

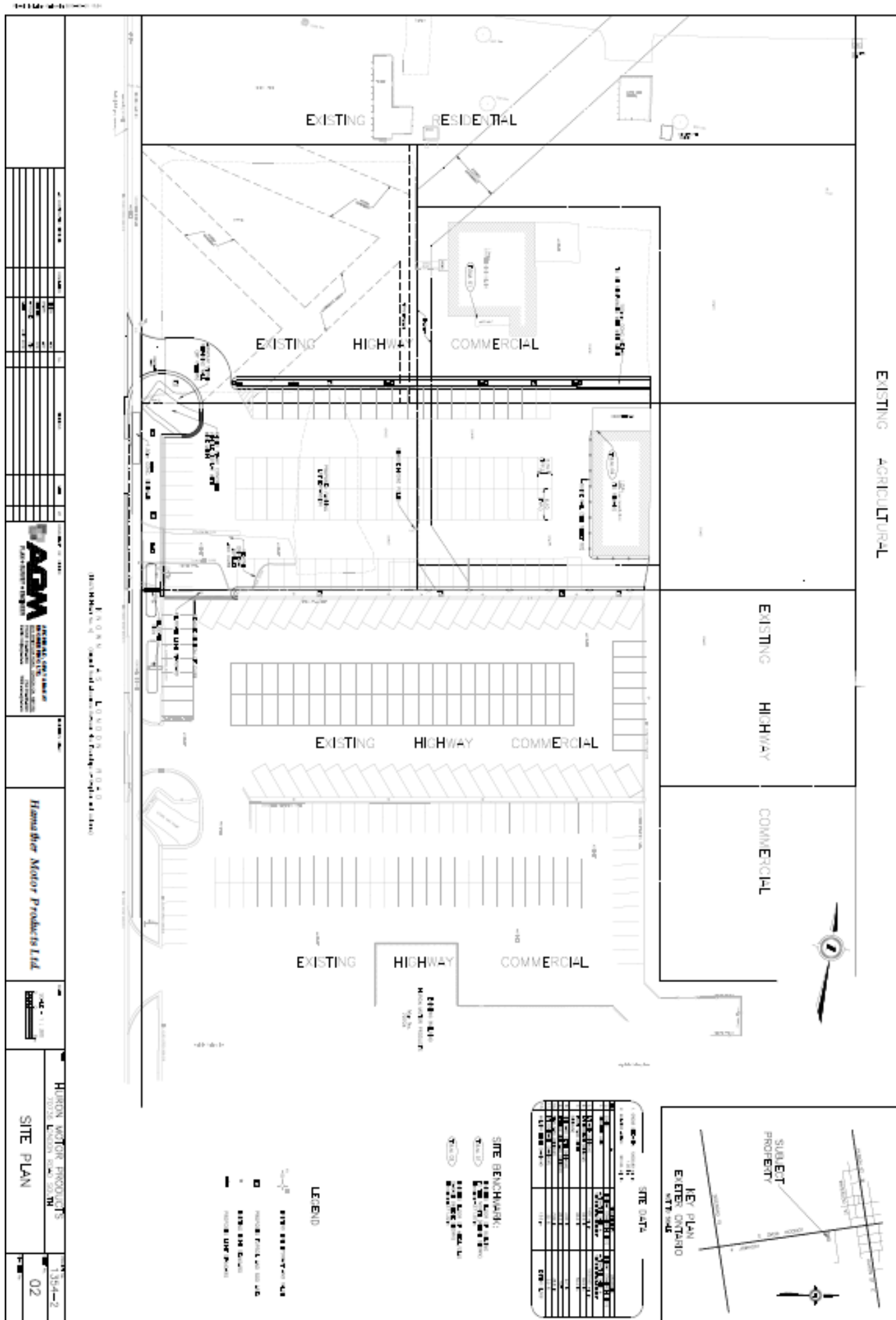
SCHEDULE "A"
LEGAL DESCRIPTION

Note: It is understood and agreed that this Schedule forms part of the Municipality's Agreement.

70736 and 70740 London Road, Exeter, ON
Roll Number 010001038000000 and 010001039000000

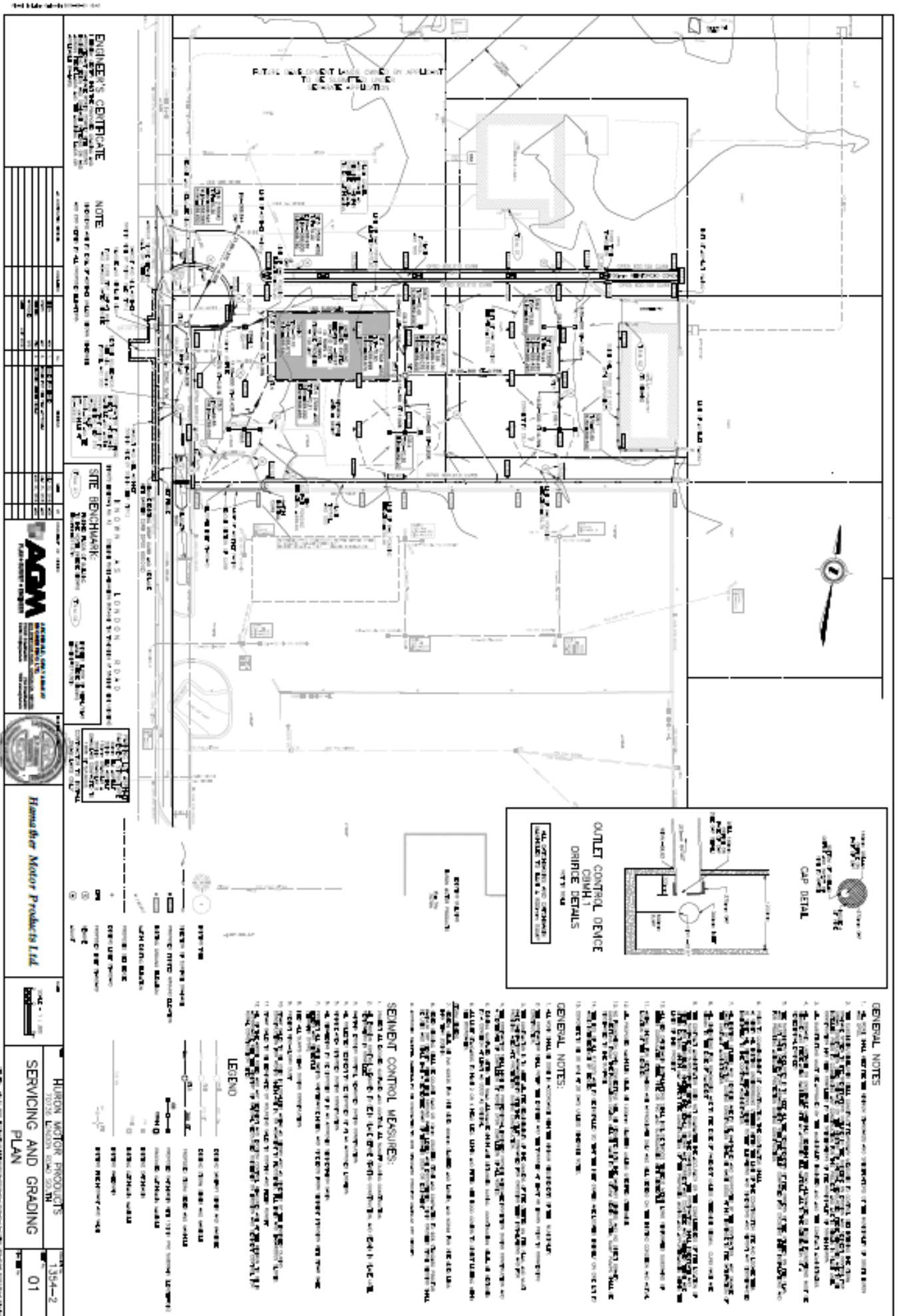
SCHEDULE "B" SITE PLAN

Note: It is understood and agreed that this Schedule forms part of the Municipality's Agreement.



SCHEDULE "C" SITE GRADING AND DRAINAGE PLAN

Note: It is understood and agreed that this Schedule forms part of the Municipality's Agreement.



SCHEDULE "D"
SECURITY TO BE PROVIDED

Note: It is understood and agreed that this Schedule forms part of the Municipality's Agreement.

ITEM	COST
Stormwater Management Plan/Storm Sewer Service	\$50,000
Sidewalks	\$2550
Roadways (Paving, Curbs, Gutter)	\$35,380
Lighting	\$10,500
Landscaping	\$5850
TOTAL	\$104,280

SCHEDULE "E"
DRAINAGE REPORT

Note: It is understood and agreed that this Schedule forms part of the Municipality's Agreement.

HAMATHER MOTOR PRODUCTS INC.

70736 LONDON ROAD SOUTH

DRAINAGE REPORT

July 31, 2018



3514 White Oak Road, London, ON N6E 2Z9 t. 519.685.5300 f. 519.685.5303 e. info@agm.on.ca

www.agm.on.ca

1.0 INTRODUCTION

The enclosed report summarizes the proposed stormwater drainage strategy for the proposed development at 70736 London Road South (Hwy.#4), in Exeter. The 1.20 ha site is being partially developed as an expansion to Huron Motor Products. The proposed vehicle storage and display lot will occupy a 0.42 ha area (Figure 1). A reduced copy of the Site Servicing and Grading Plan is attached.

2.0 STORMWATER MANAGEMENT

Stormwater Management will be provided to control post development peak discharge from the site for the 5 year through 100 year storm events.

2.1. Hydrologic Modeling

Stormwater runoff was determined by hydrologic modeling using MIDUSS (Microcomputer Interactive Design of Urban Stormwater Systems). This program allows the user to test the impact on new and existing systems, utilizing accepted rainfall data to represent design storms of various durations and aid in the design of SWM facilities.

The City of Stratford IDF curve parameters were used for the rainfall data. The 3 hour, Chicago Storm Distribution model, with a time to peak ratio of 0.38, was used for determining post development peak flow rates. These flows were used to calculate storage requirements for meeting Stormwater Management targets.

The modeling output can be found in Appendix A.

3.0 EXISTING DRAINAGE

To confirm the existing drainage patterns and outlets for the site, a topographic survey was completed. The total tributary drainage area to the location of the proposed expansion is 0.48 ha. Drainage for the area is in a westerly direction to London Road South with private catch basins draining the minor storm event to the fronting 450mm storm sewer (Figure 2).

Table 1 gives the predevelopment peak flows to London Road South.

Table 1 - Predevelopment Flows

Storm Event	Peak Flow (m ³ /s)
5 Year	0.030
100 Year	0.092

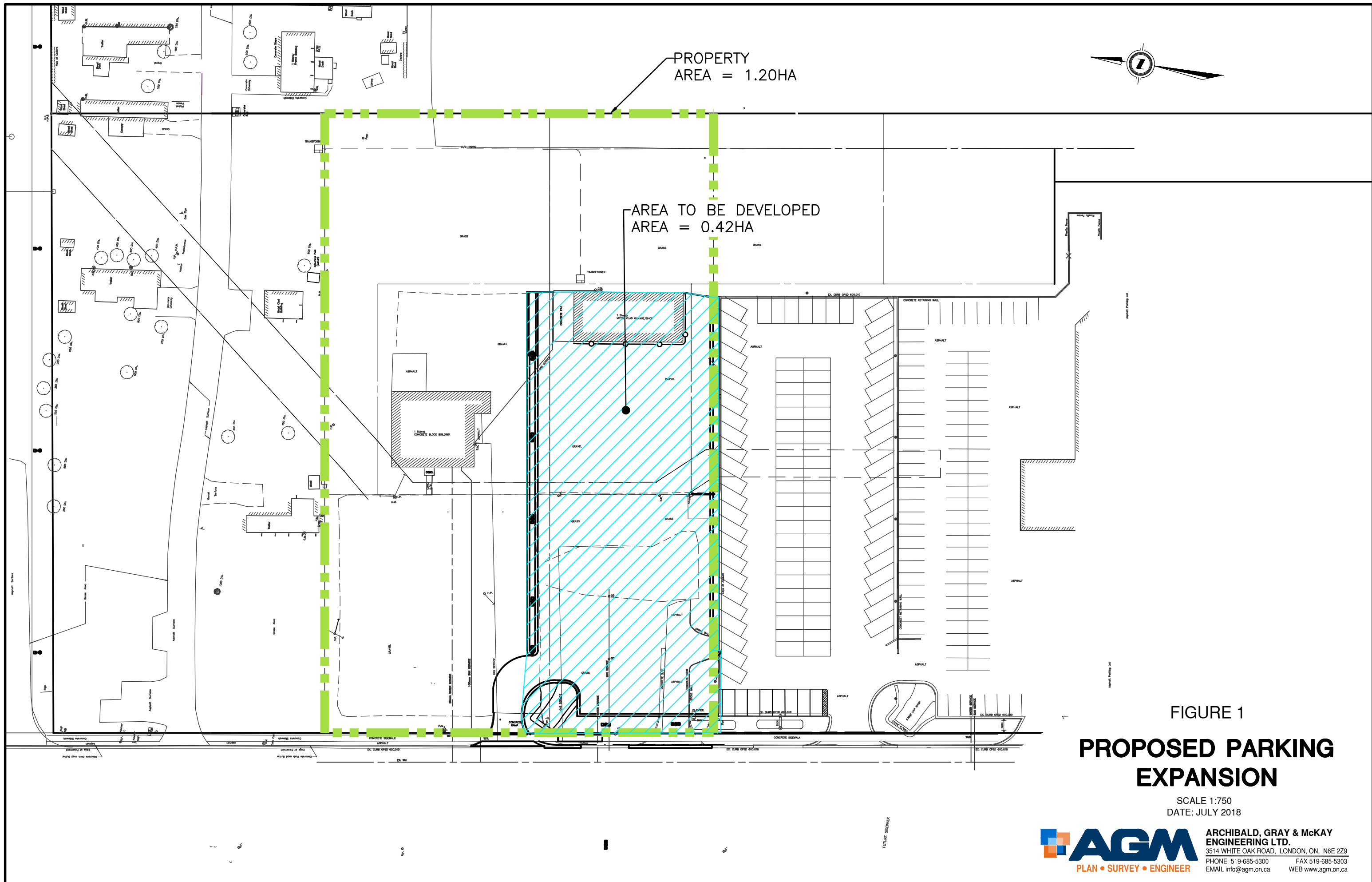
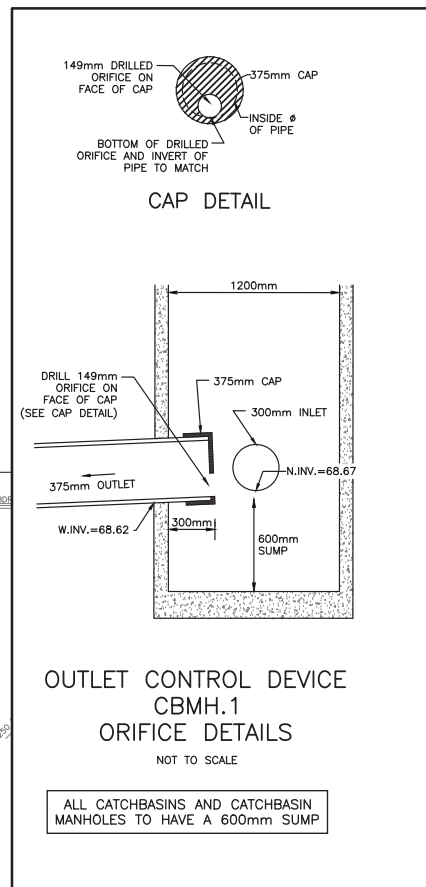


FIGURE 1
**PROPOSED PARKING
 EXPANSION**

SCALE 1:750
 DATE: JULY 2018



GENERAL NOTES

1. ALL WORK SHALL MEET THE MINIMUM STANDARDS AND SPECIFICATIONS OF THE MUNICIPALITY OF SOUTH HURON AND OPSS.
2. THE SUBDIVIDER/DEVELOPER SHALL CONSTRUCT TEMPORARY MEASURES TO CONTROL SILT ENTERING THE STORM DRAINAGE SYSTEM TO THE SPECIFICATIONS OUTLINED IN THE GUIDELINE ON EROSION AND SEDIMENT CONTROL FOR URBAN CONSTRUCTION SITES PREPARED BY THE MINISTRY OF NATURAL RESOURCES. THESE MEASURES ARE TO BE INSTALLED PRIOR TO COMMENCING ANY CONSTRUCTION FOR THIS PROJECT, AND ARE TO REMAIN IN PLACE UNTIL CONSTRUCTION HAS BEEN COMPLETED TO THE SPECIFICATIONS OF THE MUNICIPALITY OF SOUTH HURON.
3. ALL SUBSTITUTIONS MUST BE APPROVED BY THE MUNICIPALITY ENGINEER AND AND THE CONTRACT ADMINISTRATOR.
4. ALL ORGANIC, UNSTABLE OR UNSUITABLE MATERIALS BENEATH THE ROAD ALLOWANCES OR HOUSE FOUNDATIONS MUST BE REMOVED AND THESE AREAS BACKFILLED WITH AN APPROVED FILL MATERIAL, ALL TO THE SATISFACTION OF A GEOTECHNICAL ENGINEER.
5. THE SUBDIVIDER/DEVELOPER IS TO MEET ALL THE REQUIREMENTS OF THE OWNERS OF THE UTILITIES ON THIS PLAN, AND MUST MAKE SATISFACTORY ARRANGEMENTS WITH THE UTILITY COMPANIES FOR CROSSING THEIR INSTALLATIONS AND FOR PROVIDING ADEQUATE PROTECTION DURING CONSTRUCTION.
6. PRIOR TO COMMENCEMENT OF CONSTRUCTION THE CONTRACTOR SHALL:
 - A) VERIFY ALL EXISTING UNDERGROUND UTILITIES WITHIN THE LIMIT OF THE CONSTRUCTION SITE AND LOCATE THEM.
 - B) VERIFY ALL BENCHMARK ELEVATIONS, SEWER OUTLET ELEVATIONS AND DIMENSIONS AND REPORT ANY DISCREPANCIES IMMEDIATELY TO THE CONTRACT ADMINISTRATOR.
7. ALL EXISTING UTILITIES AND SERVICES ARE TO BE MAINTAINED AND SUPPORTED BY THE CONTRACTOR. ANY DAMAGE CAUSED BY THE CONTRACTOR DURING CONSTRUCTION SHALL BE MADE GOOD AT HIS EXPENSE TO THE SATISFACTION OF THE CONTRACT ADMINISTRATOR.
8. ALL DIMENSIONS FOR ROADWORKS ARE TO THE EDGE OF PAVEMENT UNLESS OTHERWISE SHOWN. CURB RADI ARE SHOWN TO THE EDGE OF PAVEMENT.
9. THE CONTRACT ADMINISTRATOR DOES NOT GUARANTEE THE ACCURACY OR THE COMPLETENESS OF THE LOCATION OF THE SURFACE OR SUBSURFACE DETAILS SHOWN ON THE DRAWINGS. THE CONTRACTOR SHALL INVESTIGATE AND VERIFY FOR THEMSELVES WHETHER THE DETAILS SHOWN ARE CORRECT AND COMPLETE AND WHETHER THERE ARE OTHER FACTORS WHICH MAY AFFECT THE CONSTRUCTION OF THE PROPOSED WORKS AND APPURTENANCES WITHIN THE LIMITS OF THIS PROJECT.
10. ALL NEW CATCHBASIN INSTALLATIONS SHALL INCLUDE TWO(2) THREE(3) METRE LONG PERFORATED SUBDRAINS OF EITHER CORRUGATED STEEL PIPE OR PVC AND SURROUNDED WITH GEOTEXTILE.
11. AREAS SHOWN FOR RESTORATION ARE APPROXIMATE ONLY AND WILL DEPEND ON THE EXISTING CONDITION AND ACTUAL LOCATION OF THE SERVICES.
12. ALL PROPOSED MANHOLES SHALL BE 1200mm DIAMETER UNLESS SPECIFIED OTHERWISE.
13. NO FOUNDATION DRAIN CONNECTIONS WILL BE PERMITTED INTO THE SANITARY SEWERS AND NO DIRECT GRAVITY CONNECTIONS FROM THE FOUNDATION DRAINS WILL BE MADE TO THE STORM SEWER SYSTEM, SUMP PUMPS SHALL BE DISCHARGED TO THE SURFACE.
14. THE LIDS OF ALL R.V.CB'S ARE TO BE INSTALLED SO THAT THE INLET GRATES ARE LOCATED ENTIRELY ON ONE LOT TO AVOID INTERFERENCE BY FENCES.
15. CONCRETE TO BE 30 MPa AT 28 DAYS UNLESS OTHERWISE NOTED.

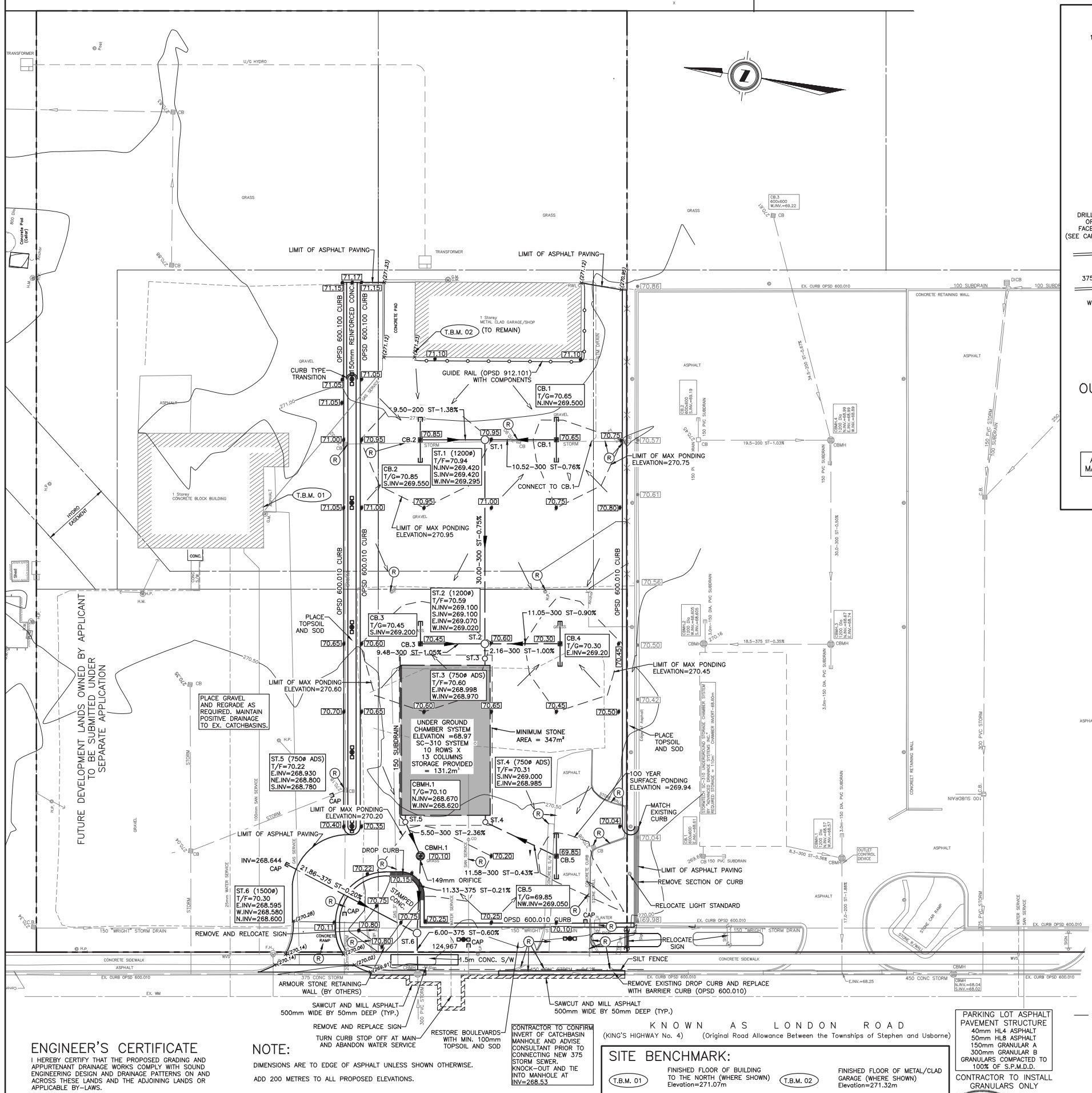
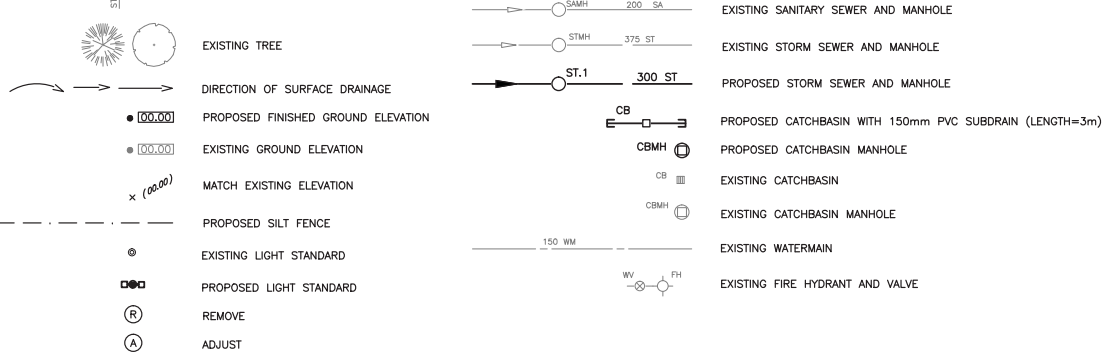
GENERAL NOTES:

1. ALL WORK SHALL BE DONE IN ACCORDANCE WITH THE MINIMUM REQUIREMENTS OF THE MUNICIPALITY.
 2. THE CONTRACTOR SHALL NOTIFY THE ENGINEER AND THE TOWNSHIP AT LEAST 48 HOURS PRIOR TO COMMENCING CONSTRUCTION.
 3. THE CONTRACTOR IS TO MEET ALL THE REQUIREMENTS OF THE OWNERS OF THE UTILITIES ON THIS PLAN, AND MUST MAKE SATISFACTORY ARRANGEMENTS WITH THE UTILITY COMPANIES FOR CROSSING THEIR INSTALLATIONS AND FOR PROVIDING ADEQUATE PROTECTION DURING CONSTRUCTION.
 4. THE CONTRACTOR SHALL HAVE ITS PROFESSIONAL ENGINEER PROVIDE ADEQUATE INSPECTION DURING CONSTRUCTION AND A CERTIFICATE OF COMPLETION OF WORKS UPON COMPLETION OF ALL WORKS.
 5. EXISTING SURFACES WITHIN THE ROAD ALLOWANCE WHICH ARE DISTURBED DURING CONSTRUCTION SHALL BE RESTORED TO A CONDITION AT LEAST AS GOOD AS ORIGINAL.
 6. ALL LIGHT STANDARDS TO HAVE 2 OR 4 HEAD L.E.D. LIGHTS AND FITTED WITH HOOD COVERS TO DIRECT LIGHTING WITHIN SITE BOUNDARIES.
- STORM SEWERS**
7. SEWERS SHALL BE PVC SDR35 FOR PIPE SIZES 200mm DIAMETER AND LARGER, AND SDR28 FOR PIPE SIZES LESS THAN 200mm.
 8. SEWER BEDDING SHALL BE COURSE SAND OR 19mm CRUSHED STONE AND COMPACTED TO 95% STANDARD PROCTOR MAXIMUM DRY DENSITY. WHERE HIGH GROUND WATER LEVELS ARE ENCOUNTERED, 19mm CRUSHED STONE BEDDING SHALL BE USED AND SHALL HAVE A GEOTEXTILE WRAP OF TERRAFIX 200R OR EQUIVALENT.
 9. APPROVED BACKFILL MATERIAL TO BE COMPACTED TO 95% STANDARD PROCTOR MAXIMUM DRY DENSITY.

SEDIMENT CONTROL MEASURES:

1. PROTECT ALL EXPOSED SURFACES AND CONTROL ALL RUNOFF DURING CONSTRUCTION.
2. ALL EROSION CONTROL MEASURES TO BE IN PLACE BEFORE STARTING CONSTRUCTION, AND REMAIN IN PLACE UNTIL RESTORATION IS COMPLETE.
3. MAINTAIN EROSION CONTROL MEASURES DURING CONSTRUCTION.
4. ALL COLLECTED SEDIMENT TO BE DISPOSED OF AT AN APPROVED LOCATION.
5. MINIMIZE AREA DISTURBED DURING CONSTRUCTION.
6. ALL DEWATERING TO BE DISPOSED OF IN AN APPROVED SEDIMENTATION BASIN.
7. PROTECT ALL CATCH BASINS, MAINTENANCE HOLES, AND PIPE ENDS FROM SEDIMENT INTRUSION WITH STRAW BALE FILTERS AND GEOTEXTILE (TERRAFIX 270R).
8. KEEP ALL SUMPS CLEAN DURING CONSTRUCTION.
9. PREVENT WIND-BLOWN DUST.
10. STRAW BALES TO BE USED IN LOCALIZED AREAS AS SHOWN AND AS DIRECTED BY THE ENGINEER DURING CONSTRUCTION FOR WORKS WHICH ARE IN, OR ADJACENT TO FLOODLINES, FILL LINES AND HAZARDOUS SLOPES.
11. STRAW BALES TO BE TERMINATED BY ROUNDING BALES TO CONTAIN AND FILTER RUNOFF.
12. ALL OF THE ABOVE NOTES AND ANY SEDIMENT AND EROSION CONTROL MEASURES ARE AT THE MINIMUM TO BE IN ACCORDANCE WITH THE MINISTRY OF NATURAL RESOURCES GUIDELINES ON EROSION AND SEDIMENT CONTROL FOR RURAL CONSTRUCTION SITES.

LEGEND



ENGINEER'S CERTIFICATE
I HEREBY CERTIFY THAT THE PROPOSED GRADING AND APPURTENANT DRAINAGE WORKS COMPLY WITH SOUND ENGINEERING DESIGN AND DRAINAGE PATTERNS ON AND ACROSS THESE LANDS AND THE ADJOINING LANDS OR APPLICABLE BY-LAWS.

NOTE:
DIMENSIONS ARE TO EDGE OF ASPHALT UNLESS SHOWN OTHERWISE.
ADD 200 METRES TO ALL PROPOSED ELEVATIONS.

CONTRACTOR TO CONFIRM INVERT OF CATCHBASIN MANHOLE AND ADVISE CONSULTANT PRIOR TO CONNECTING NEW 375 STORM SEWER. KNOCK-OUT AND TIE INTO MANHOLE AT INV=268.53

KNOWN AS LONDON ROAD
(Original Road Allowance Between the Townships of Stephen and Usborne)

SITE BENCHMARK:
(T.B.M. 01) FINISHED FLOOR OF BUILDING TO THE NORTH (WHERE SHOWN) Elevation=271.07m
(T.B.M. 02) FINISHED FLOOR OF METAL/CLAD GARAGE (WHERE SHOWN) Elevation=271.32m

PARKING LOT ASPHALT PAVEMENT STRUCTURE
40mm HL4 ASPHALT
50mm HL8 ASPHALT
150mm GRANULAR A
300mm GRANULAR B
GRANULARS COMPACTED TO 100% OF S.P.M.D.D.
CONTRACTOR TO INSTALL GRANULARS ONLY

AS CONSTRUCTED SERVICES	COMPLETION	No.	REVISIONS	DATE	BY	CONSULTANT OR DIVISION
DESIGN	AGM	1.	ISSUED FOR TENDER	JULY 23, 2018	AGM	
DRAWN	ADV	2.	ISSUED FOR SITE PLAN APPROVALS	JULY 31, 2018	AGM	
CHECKED	DTR					
APPROVED	SNS					
DATE	JULY 2018					



Hamather Motor Products Ltd.

SCALE: 1 : 300

PROJECT No. 1354-2

SHEET No. 01

PLAN FILE No.

HURON MOTOR PRODUCTS
70736 LONDON ROAD SOUTH

SERVICING AND GRADING PLAN

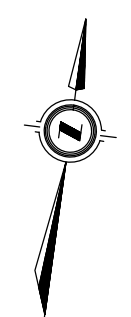
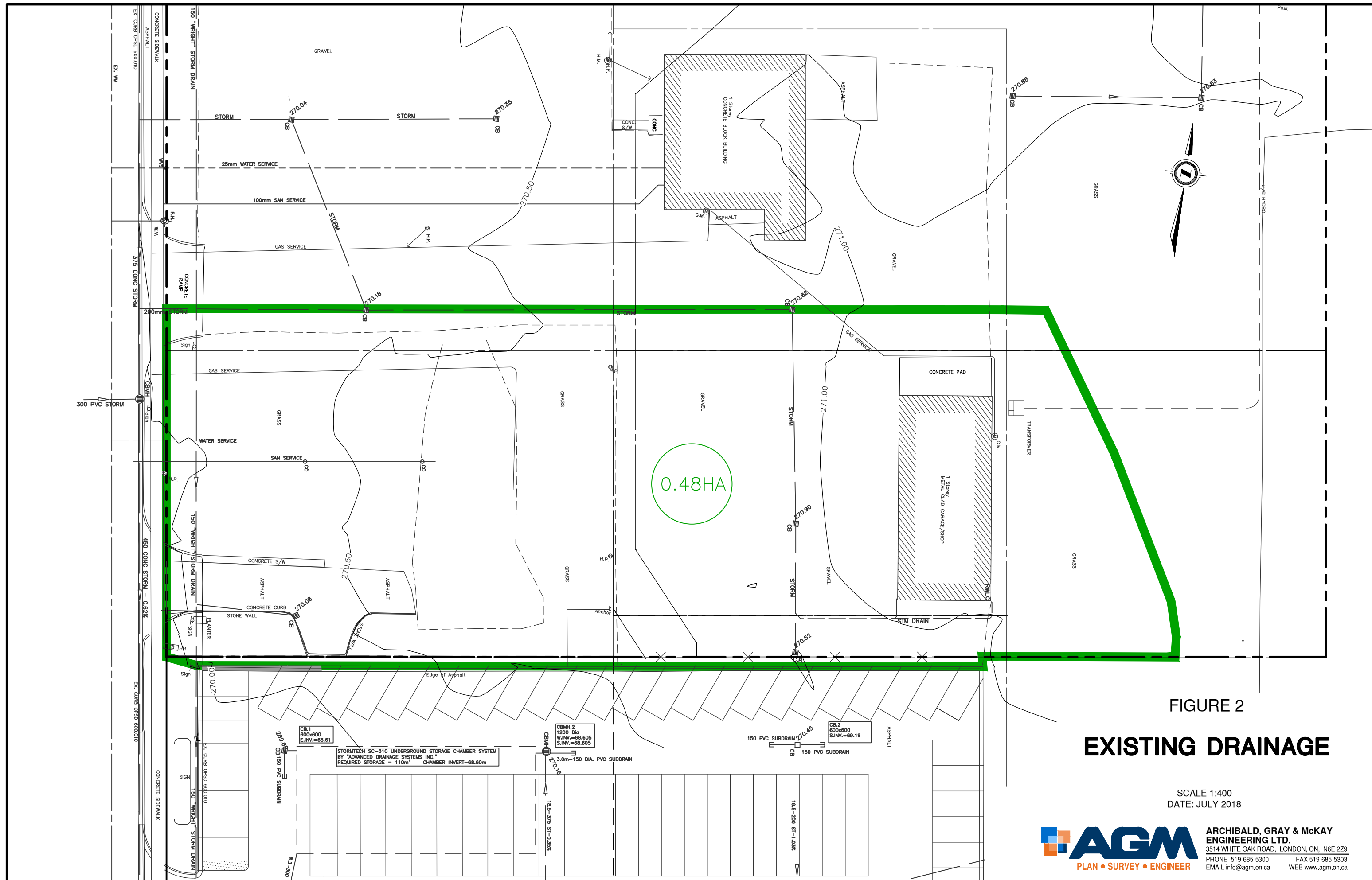


FIGURE 2

EXISTING DRAINAGE

SCALE 1:400
DATE: JULY 2018



**ARCHIBALD, GRAY & MCKAY
ENGINEERING LTD.**
3514 WHITE OAK ROAD, LONDON, ON, N6E 2Z9
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EMAIL info@agm.on.ca WEB www.agm.on.ca

4.0 PROPOSED DRAINAGE

The development of the site will increase the amount of hard surface, resulting in a 67% increase in imperviousness within the development area.

Post development drainage will be directed toward the onsite storm system which will consist of a number of catchbasins discharging through onsite storm sewers to an underground chamber, which will provide storage to limit post development peak discharge. The shop drawings for the chamber are included in Appendix B.

A 149mm diameter orifice located on the 375mm outlet pipe in catchbasin manhole CBMH.1 will control discharge to the existing storm sewer on London Road South. The orifice has been sized to limit the sewer discharge from the site to the predevelopment levels for the 5 to 100 year storm events

Post-Development peak flows and required storage volumes to control to the Pre Development peak flow rates are shown in Table 2.

Table 2 - Post Development Flows

Storm Event	Peak Flow (m ³ /s)	Storage (m ³)
5 Year	0.030	57.0
100 Year	0.054	137.5

Site storage is comprised of the underground storage system (131.2m³), upstream pipes and manholes (6.1m³), as well as surface storage (4.0m³). These combined elements will provide the required storage for all storm events up to and including the 100 year storm. Additional storage is available within catchbasins and the respective catchbasin leads on site.

5.0 SUMMARY

Site storage has been provided to control post development peak discharge to pre development levels.

Archibald, Gray & McKay Engineering Ltd.



Lukas Grabowski
Engineer-in-Training

Steve Brown, P.Eng.
Engineering Design Manager



Appendix A

Hydrologic Modeling

Pre Development

```

"          MI DUSS Output ----->"
"          MI DUSS version                               Version 2.07 rev. 387"
"          MI DUSS created                               Friday, September 23, 2005"
"          10 Units used:                                ie METRIC"
"          Job folder:                                  F:\Projects\U\usborne\US\US-01\US-01-14\
"          US-01-14-11\ENG 1354-2\ENG\SWM\MI DUSS\Stormtech Model"
"          Output filename:                             5yearPre-2.out"
"          License name:                                Igrabowski"
"          Company"
"          Date & Time last used:                       6/25/2018 at 2:08:09 PM"
" 31      TIME PARAMETERS"
"          5.000 Time Step"
"          180.000 Max. Storm length"
"          1500.000 Max. Hydrograph"
" 32      STORM Chicago storm"
"          1 Chicago storm"
"          860.460 Coefficient A"
"          7.382 Constant B"
"          0.759 Exponent C"
"          0.380 Fraction R"
"          180.000 Duration"
"          1.000 Time step multiplier"
"          Maximum intensity           118.338 mm/hr"
"          Total depth                 48.624 mm"
"          6 005hyd Hydrograph extension used in this file"
" 33      CATCHMENT 101"
"          1 Tri angular SCS"
"          3 Specify values"
"          1 SCS method"
"          101 No description"
"          16.000 % Impervious"
"          0.484 Total Area"
"          77.000 Flow length"
"          1.300 Overland Slope"
"          0.407 Pervious Area"
"          77.000 Pervious length"
"          1.300 Pervious slope"
"          0.077 Impervious Area"
"          77.000 Impervious length"
"          1.300 Impervious slope"
"          0.250 Pervious Manning 'n'"
"          88.000 Pervious SCS Curve No."
"          0.526 Pervious Runoff coefficient"
"          0.100 Pervious Ia/S coefficient"
"          3.464 Pervious Initial abstraction"
"          0.015 Impervious Manning 'n'"
"          98.000 Impervious SCS Curve No."
"          0.893 Impervious Runoff coefficient"
"          0.100 Impervious Ia/S coefficient"
"          0.518 Impervious Initial abstraction"
"          0.030 0.000 0.000 0.000 c.m/sec"
"          Catchment 101 Pervious Impervious Total Area "
"          Surface Area 0.407 0.077 0.484 hectare"
"          Time of concentration 29.905 4.254 23.681 minutes"
"          Time to Centroid 132.907 92.343 123.065 minutes"
"          Rainfall depth 48.624 48.624 48.624 mm"
"          Rainfall volume 197.69 37.65 235.34 c.m"
"          Rainfall losses 23.080 5.661 20.293 mm"
"          Runoff depth 25.544 42.963 28.331 mm"
"          Runoff volume 103.85 33.27 137.12 c.m"
"          Runoff coefficient 0.526 0.893 0.585 "
"          Maximum flow 0.026 0.020 0.030 c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4 Add Runoff "
"          0.030 0.030 0.000 0.000"

```

```

"          MI DUSS Output ----->"
"          MI DUSS version                               Version 2.07 rev. 387"
"          MI DUSS created                               Friday, September 23, 2005"
"          10 Units used:                                ie METRIC"
"          Job folder:                                  F:\Projects\U\usborne\US\US-01\US-01-14\
"          US-01-14-11\ENG 1354-2\ENG\SWM\MI DUSS\Stormtech Model"
"          Output filename:                             100yearPre-2.out"
"          License name:                                Igrabowski"
"          Company"
"          Date & Time last used:                       6/27/2018 at 2:29:49 PM"
" 31      TIME PARAMETERS"
"          5.000 Time Step"
"          180.000 Max. Storm length"
"          1500.000 Max. Hydrograph"
" 32      STORM Chicago storm"
"          1 Chicago storm"
"          1717.700 Coefficient A"
"          12.472 Constant B"
"          0.764 Exponent C"
"          0.380 Fraction R"
"          180.000 Duration"
"          1.000 Time step multiplier"
"          Maximum intensity           181.635 mm/hr"
"          Total depth                 92.643 mm"
"          6 100hyd Hydrograph extension used in this file"
" 33      CATCHMENT 101"
"          1 Tri angular SCS"
"          3 Specify values"
"          1 SCS method"
"          101 No description"
"          16.000 % Impervious"
"          0.484 Total Area"
"          77.000 Flow length"
"          1.300 Overland Slope"
"          0.407 Pervious Area"
"          77.000 Pervious length"
"          1.300 Pervious slope"
"          0.077 Impervious Area"
"          77.000 Impervious length"
"          1.300 Impervious slope"
"          0.250 Pervious Manning 'n'"
"          88.000 Pervious SCS Curve No."
"          0.693 Pervious Runoff coefficient"
"          0.100 Pervious Ia/S coefficient"
"          3.464 Pervious Initial abstraction"
"          0.015 Impervious Manning 'n'"
"          98.000 Impervious SCS Curve No."
"          0.941 Impervious Runoff coefficient"
"          0.100 Impervious Ia/S coefficient"
"          0.518 Impervious Initial abstraction"
"          0.092 0.000 0.000 0.000 c.m/sec"
"          Catchment 101 Pervious Impervious Total Area "
"          Surface Area 0.407 0.077 0.484 hectare"
"          Time of concentration 21.733 3.528 18.048 minutes"
"          Time to Centroid 119.962 90.300 113.959 minutes"
"          Rainfall depth 92.643 92.643 92.643 mm"
"          Rainfall volume 376.65 71.74 448.39 c.m"
"          Rainfall losses 28.484 7.165 25.073 mm"
"          Runoff depth 64.159 85.478 67.570 mm"
"          Runoff volume 260.84 66.19 327.04 c.m"
"          Runoff coefficient 0.693 0.941 0.733 "
"          Maximum flow 0.081 0.034 0.092 c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4 Add Runoff "
"          0.092 0.092 0.000 0.000"

```

Post Development

```

"          MI DUSS Output ----->"
"          MI DUSS version                      Version 2.07 rev. 387"
"          MI DUSS created                      Friday, September 23, 2005"
"          10  Units used:                      ie METRIC"
"          Job folder:                          F:\Projects\U\usborne\US\US-01\US-01-14\
"          US-01-14-11\ENG 1354-2\ENG\SWM\MI DUSS\Stormtech Model"
"          Output filename:                     5 year post-final-revised-3.out"
"          License name:                       Igrabowski"
"          Company"
"          Date & Time last used:               6/27/2018 at 2:10:05 PM"
" 31      TIME PARAMETERS"
"          5.000  Time Step"
"          180.000 Max. Storm length"
"          1500.000 Max. Hydrograph"
" 32      STORM Chicago storm"
"          1  Chicago storm"
"          860.460 Coefficient A"
"          7.382  Constant B"
"          0.759  Exponent C"
"          0.380  Fraction R"
"          180.000 Duration"
"          1.000  Time step multiplier"
"          Maximum intensity          118.338  mm/hr"
"          Total depth                48.624  mm"
"          6  005hyd Hydrograph extension used in this file"
" 33      CATCHMENT 1"
"          1  Tri angular SCS"
"          1  Equal length"
"          1  SCS method"
"          1  Sump #1"
"          100.000 % Impervious"
"          0.056  Total Area"
"          14.000  Flow length"
"          2.000  Overland Slope"
"          0.000  Pervious Area"
"          14.000  Pervious length"
"          2.000  Pervious slope"
"          0.056  Impervious Area"
"          14.000  Impervious length"
"          2.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          83.000  Pervious SCS Curve No."
"          0.406  Pervious Runoff coefficient"
"          0.100  Pervious Ia/S coefficient"
"          5.202  Pervious Initial abstraction"
"          0.015  Impervious Manning 'n'"
"          98.000  Impervious SCS Curve No."
"          0.893  Impervious Runoff coefficient"
"          0.100  Impervious Ia/S coefficient"
"          0.518  Impervious Initial abstraction"
"          0.014  0.000  0.000  0.000 c.m/sec"
"          Catchment 1 Pervious Impervious Total Area "
"          Surface Area 0.000 0.056 0.056 hectare"
"          Time of concentration 10.893 1.344 1.344 minutes"
"          Time to Centroid 110.712 87.966 87.966 minutes"
"          Rainfall depth 48.624 48.624 48.624 mm"
"          Rainfall volume 0.00 27.23 27.23 c.m"
"          Rainfall losses 28.907 6.069 6.069 mm"
"          Runoff depth 19.717 42.555 42.555 mm"
"          Runoff volume 0.00 23.83 23.83 c.m"
"          Runoff coefficient 0.406 0.893 0.893 "
"          Maximum flow 0.000 0.014 0.014 c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"          0.014 0.014 0.000 0.000"
" 52      CHANNEL DESIGN"
"          0.014 Current peak flow c.m/sec"
"          0.015 Manning 'n'"
"          0. Cross-section type: 0=trapezoidal; 1=general"
"          0.000 Basewidth metre"
"          50.000 Left bank slope"
"          50.000 Right bank slope"
"          1.000 Channel depth metre"
"          0.500 Gradient %"
"          Depth of flow 0.031 metre"
"          Velocity 0.293 m/sec"
"          Channel capacity 148.463 c.m/sec"
"          Critical depth 0.028 metre"
" 53      ROUTE Zero Route"
"          0.00 Zero Route Reach Length (metre)"
"          0.014 0.014 0.014 0.000 c.m/sec"

```



```

" 40      HYDROGRAPH  Combine  113"
"          6  Combine "
"          113 Node #"
"          TO ADS UNIT"
"          Maximum flow          0.014  c. m/sec"
"          Hydrograph volume     23.831  c. m"
"          0.014  0.014  0.014  0.014"
" 40      HYDROGRAPH Start - New Tributary"
"          2  Start - New Tributary"
"          0.014  0.000  0.014  0.014"
" 33      CATCHMENT 2"
"          1  Tri angular SCS"
"          3  Specify values"
"          1  SCS method"
"          2  sump #2"
"          54.500 % Impervious"
"          0.129 Total Area"
"          28.000 Flow length"
"          2.000 Overland Slope"
"          0.059 Pervious Area"
"          28.000 Pervious length"
"          2.000 Pervious slope"
"          0.070 Impervious Area"
"          15.500 Impervious length"
"          2.000 Impervious slope"
"          0.250 Pervious Manning 'n'"
"          83.000 Pervious SCS Curve No."
"          0.406 Pervious Runoff coefficient"
"          0.100 Pervious Ia/S coefficient"
"          5.202 Pervious Initial abstraction"
"          0.015 Impervious Manning 'n'"
"          98.000 Impervious SCS Curve No."
"          0.893 Impervious Runoff coefficient"
"          0.100 Impervious Ia/S coefficient"
"          0.518 Impervious Initial abstraction"
"          0.018  0.000  0.014  0.014 c. m/sec"
"          Catchment 2 Pervious Impervious Total Area "
"          Surface Area 0.059 0.070 0.129 hectare"
"          Time of concentration 16.510 1.429 5.629 minutes"
"          Time to Centroid 117.564 88.132 96.328 minutes"
"          Rainfall depth 48.624 48.624 48.624 mm"
"          Rainfall volume 28.54 34.19 62.73 c. m"
"          Rainfall losses 28.905 5.970 16.405 mm"
"          Runoff depth 19.719 42.655 32.219 mm"
"          Runoff volume 11.57 29.99 41.56 c. m"
"          Runoff coefficient 0.406 0.893 0.672 "
"          Maximum flow 0.004 0.018 0.018 c. m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"          0.018  0.018  0.014  0.014"
" 52      CHANNEL DESIGN"
"          0.018 Current peak flow c. m/sec"
"          0.015 Manning 'n'"
"          0. Cross-section type: 0=trapezoidal; 1=general "
"          0.000 Basewidth metre"
"          50.000 Left bank slope"
"          50.000 Right bank slope"
"          1.000 Channel depth metre"
"          0.500 Gradient %"
"          Depth of flow 0.034 metre"
"          Velocity 0.312 m/sec"
"          Channel capacity 148.463 c. m/sec"
"          Critical depth 0.031 metre"
" 53      ROUTE Zero Route"
"          0.00 Zero Route Reach length (metre)"
"          0.018  0.018  0.018  0.014 c. m/sec"
" 40      HYDROGRAPH  Combine  113"
"          6  Combine "
"          113 Node #"
"          TO ADS UNIT"
"          Maximum flow          0.033  c. m/sec"
"          Hydrograph volume     65.393  c. m"
"          0.018  0.018  0.018  0.033"
" 40      HYDROGRAPH Start - New Tributary"
"          2  Start - New Tributary"
"          0.018  0.000  0.018  0.033"
" 33      CATCHMENT 3"
"          1  Tri angular SCS"
"          1  Equal length"
"          1  SCS method"
"          3  sump #3"

```

```

" 100.000 % Impervious"
" 0.062 Total Area"
" 22.360 Flow Length"
" 2.000 Overland Slope"
" 0.000 Pervious Area"
" 22.360 Pervious Length"
" 2.000 Pervious slope"
" 0.062 Impervious Area"
" 22.360 Impervious Length"
" 2.000 Impervious slope"
" 0.250 Pervious Manning 'n'"
" 83.000 Pervious SCS Curve No."
" 0.406 Pervious Runoff coefficient"
" 0.100 Pervious Ia/S coefficient"
" 5.202 Pervious Initial abstraction"
" 0.015 Impervious Manning 'n'"
" 98.000 Impervious SCS Curve No."
" 0.893 Impervious Runoff coefficient"
" 0.100 Impervious Ia/S coefficient"
" 0.518 Impervious Initial abstraction"
" 0.015 0.000 0.018 0.033 c.m/sec"
" Catchment 3 Pervious Impervious Total Area "
" Surface Area 0.000 0.062 0.062 hectare"
" Time of concentration 14.426 1.780 1.780 minutes"
" Time to Centroid 115.010 88.668 88.668 minutes"
" Rainfall depth 48.624 48.624 48.624 mm"
" Rainfall volume 0.00 30.15 30.15 c.m"
" Rainfall losses 28.884 5.815 5.815 mm"
" Runoff depth 19.741 42.809 42.809 mm"
" Runoff volume 0.00 26.54 26.54 c.m"
" Runoff coefficient 0.406 0.893 0.893 "
" Maximum flow 0.000 0.015 0.015 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
" 4 Add Runoff "
" 0.015 0.015 0.018 0.033"
" 52 CHANNEL DESIGN"
" 0.015 Current peak flow c.m/sec"
" 0.015 Manning 'n'"
" 0 Cross-section type: 0=trapezoidal; 1=general"
" 0.000 Basewidth metre"
" 50.000 Left bank slope"
" 50.000 Right bank slope"
" 1.000 Channel depth metre"
" 0.500 Gradient %"
" Depth of flow 0.032 metre"
" Velocity 0.298 m/sec"
" Channel capacity 148.463 c.m/sec"
" Critical depth 0.028 metre"
" 53 ROUTE Zero Route"
" 0.00 Zero Route Reach Length (metre)"
" 0.015 0.015 0.015 0.033 c.m/sec"
" 40 HYDROGRAPH Combine 113"
" 6 Combine "
" 113 Node #"
" TO ADS UNIT"
" Maximum flow 0.048 c.m/sec"
" Hydrograph volume 91.935 c.m"
" 0.015 0.015 0.015 0.048"
" 40 HYDROGRAPH Start - New Tributary"
" 2 Start - New Tributary"
" 0.015 0.000 0.015 0.048"
" 33 CATCHMENT 4"
" 1 Triangular SCS"
" 1 Equal Length"
" 1 SCS method"
" 4 sump #4"
" 100.000 % Impervious"
" 0.060 Total Area"
" 22.130 Flow Length"
" 2.000 Overland Slope"
" 0.000 Pervious Area"
" 22.130 Pervious Length"
" 2.000 Pervious slope"
" 0.060 Impervious Area"
" 22.130 Impervious Length"
" 2.000 Impervious slope"
" 0.250 Pervious Manning 'n'"
" 83.000 Pervious SCS Curve No."
" 0.406 Pervious Runoff coefficient"
" 0.100 Pervious Ia/S coefficient"
" 5.202 Pervious Initial abstraction"

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" 0.015 Impervious Manning 'n' "
" 98.000 Impervious SCS Curve No. "
" 0.893 Impervious Runoff coefficient"
" 0.100 Impervious Ia/S coefficient"
" 0.518 Impervious Initial abstraction"
" 0.015 0.000 0.015 0.048 c.m/sec"
" Catchment 4 Pervious Impervious Total Area "
" Surface Area 0.000 0.060 0.060 hectare"
" Time of concentration 14.337 1.769 1.769 minutes"
" Time to Centroid 114.900 88.655 88.655 minutes"
" Rainfall depth 48.624 48.624 48.624 mm"
" Rainfall volume 0.00 29.17 29.17 c.m"
" Rainfall losses 28.884 5.812 5.812 mm"
" Runoff depth 19.740 42.812 42.812 mm"
" Runoff volume 0.00 25.69 25.69 c.m"
" Runoff coefficient 0.406 0.893 0.893 "
" Maximum flow 0.000 0.015 0.015 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
" 4 Add Runoff "
" 0.015 0.015 0.048"
" 52 CHANNEL DESIGN"
" 0.015 Current peak flow c.m/sec"
" 0.015 Manning 'n' "
" 0. Cross-section type: 0=trapezoidal; 1=general "
" 0.000 Basewidth metre"
" 50.000 Left bank slope"
" 50.000 Right bank slope"
" 1.000 Channel depth metre"
" 0.500 Gradient %"
" Depth of flow 0.032 metre"
" Velocity 0.298 m/sec"
" Channel capacity 148.463 c.m/sec"
" Critical depth 0.028 metre"
" 53 ROUTE Zero Route"
" 0.00 Zero Route Reach length (metre)"
" 0.015 0.015 0.015 0.048 c.m/sec"
" 40 HYDROGRAPH Combine 113"
" 6 Combine "
" 113 Node #"
" TO ADS UNIT"
" Maximum flow 0.063 c.m/sec"
" Hydrograph volume 117.622 c.m"
" 0.015 0.015 0.015 0.063"
" 40 HYDROGRAPH Start - New Tributary"
" 2 Start - New Tributary"
" 0.015 0.000 0.015 0.063"
" 33 CATCHMENT 5"
" 1 Triangular SCS"
" 1 Equal length"
" 1 SCS method"
" 5 Sump #5"
" 100.000 % Impervious"
" 0.064 Total Area"
" 23.400 Flow length"
" 2.000 Overland Slope"
" 0.000 Pervious Area"
" 23.400 Pervious length"
" 2.000 Pervious slope"
" 0.064 Impervious Area"
" 23.400 Impervious length"
" 2.000 Impervious slope"
" 0.250 Pervious Manning 'n' "
" 83.000 Pervious SCS Curve No. "
" 0.406 Pervious Runoff coefficient"
" 0.100 Pervious Ia/S coefficient"
" 5.202 Pervious Initial abstraction"
" 0.015 Impervious Manning 'n' "
" 98.000 Impervious SCS Curve No. "
" 0.893 Impervious Runoff coefficient"
" 0.100 Impervious Ia/S coefficient"
" 0.518 Impervious Initial abstraction"
" 0.016 0.000 0.015 0.063 c.m/sec"
" Catchment 5 Pervious Impervious Total Area "
" Surface Area 0.000 0.064 0.064 hectare"
" Time of concentration 14.825 1.829 1.829 minutes"
" Time to Centroid 115.494 88.713 88.713 minutes"
" Rainfall depth 48.624 48.624 48.624 mm"
" Rainfall volume 0.00 31.12 31.12 c.m"
" Rainfall losses 28.895 5.824 5.824 mm"
" Runoff depth 19.729 42.800 42.800 mm"
" Runoff volume 0.00 27.39 27.39 c.m"

```

```

"      Runoff coefficient      0.406      0.893      0.893      "
"      Maximum flow            0.000      0.016      0.016      c. m/sec"
" 40  HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"          0.016      0.016      0.063"
" 52  CHANNEL DESIGN"
"      0.016  Current peak flow  c. m/sec"
"      0.015  Manning 'n'"
"      0.      Cross-section type: 0=trapezoidal ; 1=general "
"      0.600  Basewidth  metre"
"      50.000 Left bank slope"
"      50.000 Right bank slope"
"      1.000  Channel depth  metre"
"      0.500  Gradient  %"
"          Depth of flow            0.027  metre"
"          Velocity                  0.300  m/sec"
"          Channel capacity          150.842 c. m/sec"
"          Critical depth            0.024  metre"
" 53  ROUTE Zero Route"
"      0.00  Zero Route Reach length ( metre)"
"          0.016      0.016      0.016      0.063 c. m/sec"
" 40  HYDROGRAPH Combine 113"
"      6  Combine "
"      113 Node #"
"          TO ADS UNIT"
"          Maximum flow              0.078  c. m/sec"
"          Hydrograph volume         145.014 c. m"
"          0.016      0.016      0.016      0.078"
" 40  HYDROGRAPH Start - New Tributary"
"      2  Start - New Tributary"
"          0.016      0.000      0.016      0.078"
" 33  CATCHMENT 6"
"      1  Triangular SCS"
"      1  Equal length"
"      1  SCS method"
"      6  sump #6"
"      100.000 % Impervious"
"      0.065  Total Area"
"      23.300 Flow length"
"      2.000  Overland Slope"
"      0.000  Pervious Area"
"      23.300 Pervious length"
"      2.000  Pervious slope"
"      0.065  Impervious Area"
"      23.300 Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"      83.000 Pervious SCS Curve No."
"      0.406  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      5.202  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.893  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.016      0.000      0.016      0.078 c. m/sec"
"          Catchment 6      Pervious  Impervious Total Area "
"          Surface Area      0.000      0.065      0.065  hectare"
"          Time of concentration  14.787      1.825      1.825  minutes"
"          Time to Centroid      115.447      88.710      88.710  minutes"
"          Rainfall depth      48.624      48.624      48.624  mm"
"          Rainfall volume      0.00      31.61      31.61  c. m"
"          Rainfall losses      28.893      5.824      5.824  mm"
"          Runoff depth      19.731      42.800      42.800  mm"
"          Runoff volume      0.00      27.82      27.82  c. m"
"          Runoff coefficient    0.406      0.893      0.893  "
"          Maximum flow        0.000      0.016      0.016  c. m/sec"
" 40  HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"          0.016      0.016      0.078"
" 52  CHANNEL DESIGN"
"      0.016  Current peak flow  c. m/sec"
"      0.015  Manning 'n'"
"      0.      Cross-section type: 0=trapezoidal ; 1=general "
"      0.600  Basewidth  metre"
"      50.000 Left bank slope"
"      50.000 Right bank slope"
"      1.000  Channel depth  metre"
"      0.500  Gradient  %"
"          Depth of flow            0.027  metre"

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"      Velocity                0.300    m/sec"
"      Channel capacity        150.842  c. m/sec"
"      Critical depth          0.024    metre"
" 53  ROUTE Zero Route"
"      0.00 Zero Route Reach Length (metre)"
"            0.016    0.016    0.016    0.078 c. m/sec"
" 40  HYDROGRAPH Combine 113"
"      6 Combine "
"      113 Node #"
"      TO ADS UNIT"
"      Maximum flow            0.094    c. m/sec"
"      Hydrograph volume       172.835  c. m"
"            0.016    0.016    0.016    0.094"
" 40  HYDROGRAPH Confluence 113"
"      7 Confluence "
"      113 Node #"
"      TO ADS UNIT"
"      Maximum flow            0.094    c. m/sec"
"      Hydrograph volume       172.835  c. m"
"            0.016    0.094    0.016    0.000"
" 54  POND DESIGN"
"      0.094 Current peak flow c. m/sec"
"      0.012 Target outflow c. m/sec"
"      180.0 Hydrograph volume c. m"
"      46. Number of stages"
"      268.818 Minimum water level metre"
"      269.961 Maximum water level metre"
"      268.818 Starting water level metre"
"      0 Keep Design Data: 1 = True; 0 = False"
"      Level Discharge Volume"
"      268.818 0.000 0.0"
"      268.843 0.017 3.6"
"      268.869 0.019 7.1"
"      268.894 0.020 10.6"
"      268.920 0.022 14.1"
"      268.945 0.023 17.6"
"      268.970 0.024 21.0"
"      268.996 0.026 27.7"
"      269.021 0.027 34.3"
"      269.047 0.028 40.9"
"      269.072 0.029 47.4"
"      269.097 0.030 53.8"
"      269.123 0.031 60.1"
"      269.148 0.032 66.2"
"      269.174 0.033 72.3"
"      269.199 0.034 78.2"
"      269.224 0.035 84.0"
"      269.250 0.035 89.5"
"      269.275 0.036 94.8"
"      269.301 0.037 99.7"
"      269.326 0.038 104.1"
"      269.351 0.039 108.2"
"      269.377 0.039 112.0"
"      269.402 0.040 115.7"
"      269.428 0.041 119.4"
"      269.453 0.042 123.1"
"      269.478 0.042 126.7"
"      269.504 0.043 130.3"
"      269.529 0.044 133.9"
"      269.555 0.044 134.1"
"      269.580 0.045 134.1"
"      269.605 0.046 134.2"
"      269.631 0.046 134.3"
"      269.656 0.047 134.4"
"      269.682 0.048 134.5"
"      269.707 0.048 134.6"
"      269.732 0.049 134.7"
"      269.758 0.050 134.8"
"      269.783 0.050 134.9"
"      269.809 0.051 135.0"
"      269.834 0.051 135.1"
"      269.859 0.052 135.2"
"      269.885 0.053 135.4"
"      269.910 0.053 136.1"
"      269.936 0.054 137.3"
"      269.961 0.054 139.5"
" 1.  ORIFICES"
"      Ori fi ce Ori fi ce Ori fi ce Number of"
"      invert coeffi cie di ameter ori fi ces"
"      268.620 0.630 0.1490 1.000"
"      Peak outflow 0.030 c. m/sec"

```

"	Maximum level		269.110	metre"
"	Maximum storage		57.031	c. m"
"	Centroidal lag		1.812	hours"
"	0.016	0.094	0.030	0.000 c. m/sec"

```

"          MI DUSS Output ----->"
"          MI DUSS version                               Version 2.07 rev. 387"
"          MI DUSS created                               Friday, September 23, 2005"
"          10 Units used:                               ie METRIC"
"          Job folder:                                  F:\Projects\U\usborne\US\US-01\US-01-14\
"          US-01-14-11\ENG 1354-2\ENG\SWM\MI DUSS\Stormtech Model"
"          Output filename:                             100 year post-final-revised-6.out"
"          License name:                                Igrabowski"
"          Company"
"          Date & Time last used:                       6/27/2018 at 2:12:13 PM"
" 31      TIME PARAMETERS"
"          5.000 Time Step"
"          180.000 Max. Storm length"
"          1500.000 Max. Hydrograph"
" 32      STORM Chicago storm"
"          1 Chicago storm"
"          1717.700 Coefficient A"
"          12.472 Constant B"
"          0.764 Exponent C"
"          0.380 Fraction R"
"          180.000 Duration"
"          1.000 Time step multiplier"
"          Maximum intensity           181.635 mm/hr"
"          Total depth                 92.643 mm"
"          6 100hyd Hydrograph extension used in this file"
" 33      CATCHMENT 1"
"          1 Tri angular SCS"
"          1 Equal length"
"          1 SCS method"
"          1 Sump #1"
"          100.000 % Impervious"
"          0.056 Total Area"
"          14.000 Flow length"
"          2.000 Overland Slope"
"          0.000 Pervious Area"
"          14.000 Pervious length"
"          2.000 Pervious slope"
"          0.056 Impervious Area"
"          14.000 Impervious length"
"          2.000 Impervious slope"
"          0.250 Pervious Manning 'n'"
"          83.000 Pervious SCS Curve No."
"          0.592 Pervious Runoff coefficient"
"          0.100 Pervious Ia/S coefficient"
"          5.202 Pervious Initial abstraction"
"          0.015 Impervious Manning 'n'"
"          98.000 Impervious SCS Curve No."
"          0.941 Impervious Runoff coefficient"
"          0.100 Impervious Ia/S coefficient"
"          0.518 Impervious Initial abstraction"
"          0.024 0.000 0.000 0.000 c. m/sec"
"          Catchment 1 Pervious Impervious Total Area "
"          Surface Area 0.000 0.056 0.056 hectare"
"          Time of concentration 7.461 1.115 1.115 minutes"
"          Time to Centroid 104.064 86.722 86.722 minutes"
"          Rainfall depth 92.643 92.643 92.643 mm"
"          Rainfall volume 0.00 51.88 51.88 c. m"
"          Rainfall losses 38.070 7.639 7.639 mm"
"          Runoff depth 54.573 85.003 85.003 mm"
"          Runoff volume 0.00 47.60 47.60 c. m"
"          Runoff coefficient 0.592 0.941 0.941 "
"          Maximum flow 0.000 0.024 0.024 c. m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4 Add Runoff "
"          0.024 0.024 0.000 0.000"
" 52      CHANNEL DESIGN"
"          0.024 Current peak flow c. m/sec"
"          0.015 Manning 'n'"
"          0. Cross-section type: 0=trapezoidal; 1=general "
"          0.000 Basewidth metre"
"          50.000 Left bank slope"
"          50.000 Right bank slope"
"          1.000 Channel depth metre"
"          0.500 Gradient %"
"          Depth of flow 0.038 metre"
"          Velocity 0.335 m/sec"
"          Channel capacity 148.463 c. m/sec"
"          Critical depth 0.034 metre"
" 53      ROUTE Zero Route"
"          0.00 Zero Route Reach length (metre)"
"          0.024 0.024 0.024 0.000 c. m/sec"

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" 40      HYDROGRAPH  Combi ne  113"
"          6  Combi ne "
"          113 Node #"
"          TO ADS UNIT"
"          Maximum flow          0.024  c. m/sec"
"          Hydrograph volume     47.602  c. m"
"          0.024  0.024  0.024  0.024"
" 40      HYDROGRAPH Start - New Tributary"
"          2  Start - New Tributary"
"          0.024  0.000  0.024  0.024"
" 33      CATCHMENT 2"
"          1  Tri angular SCS"
"          3  Speci fy values"
"          1  SCS method"
"          2  sump #2"
"          54.500 % Impervious"
"          0.129 Total Area"
"          28.000 Flow length"
"          2.000 Overland Slope"
"          0.059 Pervious Area"
"          28.000 Pervious length"
"          2.000 Pervious slope"
"          0.070 Impervious Area"
"          15.500 Impervious length"
"          2.000 Impervious slope"
"          0.250 Pervious Manning 'n'"
"          83.000 Pervious SCS Curve No."
"          0.592 Pervious Runoff coefficient"
"          0.100 Pervious Ia/S coefficient"
"          5.202 Pervious Initial abstraction"
"          0.015 Impervious Manning 'n'"
"          98.000 Impervious SCS Curve No."
"          0.941 Impervious Runoff coefficient"
"          0.100 Impervious Ia/S coefficient"
"          0.518 Impervious Initial abstraction"
"          0.036  0.000  0.024  0.024 c. m/sec"
"          Catchment 2 Pervious Impervious Total Area "
"          Surface Area 0.059 0.070 0.129 hectare"
"          Time of concentration 11.308 1.185 4.721 minutes"
"          Time to Centroid 108.672 86.734 94.396 minutes"
"          Rainfall depth 92.643 92.643 92.643 mm"
"          Rainfall volume 54.38 65.13 119.51 c. m"
"          Rainfall losses 37.901 7.490 21.327 mm"
"          Runoff depth 54.742 85.153 71.316 mm"
"          Runoff volume 32.13 59.87 92.00 c. m"
"          Runoff coefficient 0.592 0.941 0.782 "
"          Maximum flow 0.013 0.030 0.036 c. m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"          0.036  0.036  0.024  0.024"
" 52      CHANNEL DESIGN"
"          0.036 Current peak flow c. m/sec"
"          0.015 Manning 'n'"
"          0. Cross-section type: 0=trapezoidal; 1=general "
"          0.000 Basewidth metre"
"          50.000 Left bank slope"
"          50.000 Right bank slope"
"          1.000 Channel depth metre"
"          0.500 Gradient %"
"          Depth of flow 0.044 metre"
"          Velocity 0.371 m/sec"
"          Channel capacity 148.463 c. m/sec"
"          Critical depth 0.040 metre"
" 53      ROUTE Zero Route"
"          0.00 Zero Route Reach Length (metre)"
"          0.036  0.036  0.036  0.024 c. m/sec"
" 40      HYDROGRAPH  Combi ne  113"
"          6  Combi ne "
"          113 Node #"
"          TO ADS UNIT"
"          Maximum flow          0.058  c. m/sec"
"          Hydrograph volume     139.600  c. m"
"          0.036  0.036  0.036  0.058"
" 40      HYDROGRAPH Start - New Tributary"
"          2  Start - New Tributary"
"          0.036  0.000  0.036  0.058"
" 33      CATCHMENT 3"
"          1  Tri angular SCS"
"          1  Equal length"
"          1  SCS method"
"          3  sump #3"

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" 100.000 % Impervious"
" 0.062 Total Area"
" 22.360 Flow Length"
" 2.000 Overland Slope"
" 0.000 Pervious Area"
" 22.360 Pervious Length"
" 2.000 Pervious slope"
" 0.062 Impervious Area"
" 22.360 Impervious Length"
" 2.000 Impervious slope"
" 0.250 Pervious Manning 'n'"
" 83.000 Pervious SCS Curve No."
" 0.592 Pervious Runoff coefficient"
" 0.100 Pervious Ia/S coefficient"
" 5.202 Pervious Initial abstraction"
" 0.015 Impervious Manning 'n'"
" 98.000 Impervious SCS Curve No."
" 0.941 Impervious Runoff coefficient"
" 0.100 Impervious Ia/S coefficient"
" 0.518 Impervious Initial abstraction"
" 0.026 0.000 0.036 0.058 c.m/sec"
" Catchment 3 Pervious Impervious Total Area "
" Surface Area 0.000 0.062 0.062 hectare"
" Time of concentration 9.881 1.476 1.476 minutes"
" Time to Centroid 106.998 87.229 87.229 minutes"
" Rainfall depth 92.643 92.643 92.643 mm"
" Rainfall volume 0.00 57.44 57.44 c.m"
" Rainfall losses 38.152 6.942 6.942 mm"
" Runoff depth 54.491 85.700 85.700 mm"
" Runoff volume 0.00 53.13 53.13 c.m"
" Runoff coefficient 0.592 0.941 0.941 "
" Maximum flow 0.000 0.026 0.026 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
" 4 Add Runoff "
" 0.026 0.026 0.036 0.058"
" 52 CHANNEL DESIGN"
" 0.026 Current peak flow c.m/sec"
" 0.015 Manning 'n'"
" 0. Cross-section type: 0=trapezoidal; 1=general"
" 0.000 Basewidth metre"
" 50.000 Left bank slope"
" 50.000 Right bank slope"
" 1.000 Channel depth metre"
" 0.500 Gradient %"
" Depth of flow 0.039 metre"
" Velocity 0.342 m/sec"
" Channel capacity 148.463 c.m/sec"
" Critical depth 0.035 metre"
" 53 ROUTE Zero Route"
" 0.00 Zero Route Reach Length (metre)"
" 0.026 0.026 0.026 0.058 c.m/sec"
" 40 HYDROGRAPH Combine 113"
" 6 Combine "
" 113 Node #"
" TO ADS UNIT"
" Maximum flow 0.084 c.m/sec"
" Hydrograph volume 192.734 c.m"
" 0.026 0.026 0.026 0.084"
" 40 HYDROGRAPH Start - New Tributary"
" 2 Start - New Tributary"
" 0.026 0.000 0.026 0.084"
" 33 CATCHMENT 4"
" 1 Triangular SCS"
" 1 Equal Length"
" 1 SCS method"
" 4 sump #4"
" 100.000 % Impervious"
" 0.060 Total Area"
" 22.130 Flow Length"
" 2.000 Overland Slope"
" 0.000 Pervious Area"
" 22.130 Pervious Length"
" 2.000 Pervious slope"
" 0.060 Impervious Area"
" 22.130 Impervious Length"
" 2.000 Impervious slope"
" 0.250 Pervious Manning 'n'"
" 83.000 Pervious SCS Curve No."
" 0.592 Pervious Runoff coefficient"
" 0.100 Pervious Ia/S coefficient"
" 5.202 Pervious Initial abstraction"

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"      0.015 Impervious Manning 'n' "
" 98.000 Impervious SCS Curve No. "
"      0.941 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"              0.025      0.000      0.026      0.084 c. m/sec"
"      Catchment 4      Pervious      Impervious      Total Area "
"      Surface Area      0.000      0.060      0.060      hectare"
"      Time of concentration      9.819      1.467      1.467      minutes"
"      Time to Centroid      106.919      87.211      87.211      minutes"
"      Rainfall depth      92.643      92.643      92.643      mm"
"      Rainfall volume      0.00      55.59      55.59      c. m"
"      Rainfall losses      38.149      6.956      6.956      mm"
"      Runoff depth      54.494      85.686      85.686      mm"
"      Runoff volume      0.00      51.41      51.41      c. m"
"      Runoff coefficient      0.592      0.941      0.941      "
"      Maximum flow      0.000      0.025      0.025      c. m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"              0.025      0.025      0.026      0.084"
" 52 CHANNEL DESIGN"
"      0.025 Current peak flow      c. m/sec"
"      0.015 Manning 'n' "
"      0. Cross-section type: 0=trapezoidal ; 1=general "
"      0.000 Basewidth      metre"
"      50.000 Left bank slope"
"      50.000 Right bank slope"
"      1.000 Channel depth      metre"
"      0.500 Gradient      %"
"      Depth of flow      0.038      metre"
"      Velocity      0.338      m/sec"
"      Channel capacity      148.463      c. m/sec"
"      Critical depth      0.035      metre"
" 53 ROUTE Zero Route"
"      0.00 Zero Route Reach length      ( metre)"
"              0.025      0.025      0.025      0.084 c. m/sec"
" 40 HYDROGRAPH Combine 113"
"      6 Combine "
"      113 Node #"
"      TO ADS UNIT"
"      Maximum flow      0.109      c. m/sec"
"      Hydrograph volume      244.146      c. m"
"              0.025      0.025      0.025      0.109"
" 40 HYDROGRAPH Start - New Tributary"
"      2 Start - New Tributary"
"              0.025      0.000      0.025      0.109"
" 33 CATCHMENT 5"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      5 Sump #5"
"      100.000 % Impervious"
"      0.064 Total Area"
"      23.400 Flow length"
"      2.000 Overland Slope"
"      0.000 Pervious Area"
"      23.400 Pervious length"
"      2.000 Pervious slope"
"      0.064 Impervious Area"
"      23.400 Impervious length"
"      2.000 Impervious slope"
"      0.250 Pervious Manning 'n' "
"      83.000 Pervious SCS Curve No. "
"      0.592 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      5.202 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n' "
"      98.000 Impervious SCS Curve No. "
"      0.941 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"              0.027      0.000      0.025      0.109 c. m/sec"
"      Catchment 5      Pervious      Impervious      Total Area "
"      Surface Area      0.000      0.064      0.064      hectare"
"      Time of concentration      10.154      1.517      1.517      minutes"
"      Time to Centroid      107.305      87.308      87.308      minutes"
"      Rainfall depth      92.643      92.643      92.643      mm"
"      Rainfall volume      0.00      59.29      59.29      c. m"
"      Rainfall losses      38.091      6.878      6.878      mm"
"      Runoff depth      54.551      85.765      85.765      mm"
"      Runoff volume      0.00      54.89      54.89      c. m"

```

"	Runoff coefficient	0.592	0.941	0.941	"
"	Maximum flow	0.000	0.027	0.027	c. m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.027	0.027	0.025	0.109"	
" 52	CHANNEL DESIGN"				
"	0.027 Current peak flow	c. m/sec"			
"	0.015 Manning 'n' "				
"	0. Cross-section type: 0=trapezoidal; 1=general "				
"	0.600 Basewidth	metre"			
"	50.000 Left bank slope"				
"	50.000 Right bank slope"				
"	1.000 Channel depth	metre"			
"	0.500 Gradient	%"			
"	Depth of flow	0.034	metre"		
"	Velocity	0.343	m/sec"		
"	Channel capacity	150.842	c. m/sec"		
"	Critical depth	0.030	metre"		
" 53	ROUTE Zero Route"				
"	0.00 Zero Route Reach length	(metre)"			
"	0.027	0.027	0.027	0.109	c. m/sec"
" 40	HYDROGRAPH Combine	113"			
"	6 Combine "				
"	113 Node #"				
"	TO ADS UNIT"				
"	Maximum flow	0.136	c. m/sec"		
"	Hydrograph volume	299.035	c. m"		
"	0.027	0.027	0.027	0.136"	
" 40	HYDROGRAPH Start - New Tributary"				
"	2 Start - New Tributary"				
"	0.027	0.000	0.027	0.136"	
" 33	CATCHMENT 6"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	6 sump #6"				
"	100.000 % Impervious"				
"	0.065 Total Area"				
"	23.300 Flow length"				
"	2.000 Overland Slope"				
"	0.000 Pervious Area"				
"	23.300 Pervious length"				
"	2.000 Pervious slope"				
"	0.065 Impervious Area"				
"	23.300 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n' "				
"	83.000 Pervious SCS Curve No. "				
"	0.592 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	5.202 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n' "				
"	98.000 Impervious SCS Curve No. "				
"	0.941 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"	0.027	0.000	0.027	0.136	c. m/sec"
"	Catchment 6	Pervious	Impervious	Total Area	"
"	Surface Area	0.000	0.065	0.065	hectare"
"	Time of concentration	10.128	1.513	1.513	minutes"
"	Time to Centroid	107.275	87.301	87.301	minutes"
"	Rainfall depth	92.643	92.643	92.643	mm"
"	Rainfall volume	0.00	60.22	60.22	c. m"
"	Rainfall losses	38.096	6.883	6.883	mm"
"	Runoff depth	54.547	85.760	85.760	mm"
"	Runoff volume	0.00	55.74	55.74	c. m"
"	Runoff coefficient	0.592	0.941	0.941	"
"	Maximum flow	0.000	0.027	0.027	c. m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.027	0.027	0.027	0.136"	
" 52	CHANNEL DESIGN"				
"	0.027 Current peak flow	c. m/sec"			
"	0.015 Manning 'n' "				
"	0. Cross-section type: 0=trapezoidal; 1=general "				
"	0.600 Basewidth	metre"			
"	50.000 Left bank slope"				
"	50.000 Right bank slope"				
"	1.000 Channel depth	metre"			
"	0.500 Gradient	%"			
"	Depth of flow	0.034	metre"		

```

"      Velocity                0.343    m/sec"
"      Channel capacity        150.842  c. m/sec"
"      Critical depth          0.030    metre"
" 53  ROUTE Zero Route"
"      0.00 Zero Route Reach Length (metre)"
"      0.027 0.027 0.027 0.027 0.136 c. m/sec"
" 40  HYDROGRAPH Combine 113"
"      6 Combine "
"      113 Node #"
"      TO ADS UNIT"
"      Maximum flow            0.164    c. m/sec"
"      Hydrograph volume       354.779  c. m"
"      0.027 0.027 0.027 0.164"
" 40  HYDROGRAPH Confluence 113"
"      7 Confluence "
"      113 Node #"
"      TO ADS UNIT"
"      Maximum flow            0.164    c. m/sec"
"      Hydrograph volume       354.779  c. m"
"      0.027 0.164 0.027 0.000"
" 54  POND DESIGN"
"      0.164 Current peak flow    c. m/sec"
"      0.012 Target outflow      c. m/sec"
"      355.0 Hydrograph volume    c. m"
"      46. Number of stages"
"      268.818 Minimum water level  metre"
"      269.961 Maximum water level  metre"
"      268.818 Starting water level  metre"
"      0 Keep Design Data: 1 = True; 0 = False"
"      Level Discharge Volume"
"      268.818 0.000 0.0"
"      268.843 0.017 3.6"
"      268.869 0.019 7.1"
"      268.894 0.020 10.6"
"      268.920 0.022 14.1"
"      268.945 0.023 17.6"
"      268.970 0.024 21.0"
"      268.996 0.026 27.7"
"      269.021 0.027 34.3"
"      269.047 0.028 40.9"
"      269.072 0.029 47.4"
"      269.097 0.030 53.8"
"      269.123 0.031 60.1"
"      269.148 0.032 66.2"
"      269.174 0.033 72.3"
"      269.199 0.034 78.2"
"      269.224 0.035 84.0"
"      269.250 0.035 89.5"
"      269.275 0.036 94.8"
"      269.301 0.037 99.7"
"      269.326 0.038 104.1"
"      269.351 0.039 108.2"
"      269.377 0.039 112.0"
"      269.402 0.040 115.7"
"      269.428 0.041 119.4"
"      269.453 0.042 123.1"
"      269.478 0.042 126.7"
"      269.504 0.043 130.3"
"      269.529 0.044 133.9"
"      269.555 0.044 134.1"
"      269.580 0.045 134.1"
"      269.605 0.046 134.2"
"      269.631 0.046 134.3"
"      269.656 0.047 134.4"
"      269.682 0.048 134.5"
"      269.707 0.048 134.6"
"      269.732 0.049 134.7"
"      269.758 0.050 134.8"
"      269.783 0.050 134.9"
"      269.809 0.051 135.0"
"      269.834 0.051 135.1"
"      269.859 0.052 135.2"
"      269.885 0.053 135.4"
"      269.910 0.053 136.1"
"      269.936 0.054 137.3"
"      269.961 0.054 139.5"
" 1.  ORIFICES"
"      Ori fi ce Ori fi ce Ori fi ce Number of"
"      invert coeffi cie di ameter ori fi ces"
"      268.620 0.630 0.1490 1.000"
"      Peak outflow 0.054 c. m/sec"

```

"	Maximum level		269.939	metre"
"	Maximum storage		137.530	c. m"
"	Centroidal lag		2.063	hours"
"	0.027	0.164	0.054	0.000 c. m/sec"

Appendix B
Stormwater Chamber

SWM STORAGE - UNDERGROUND CHAMBER UNIT AND UPSTREAM PIPES

Elevation (m)												Description		
	CBMH.1 (m ³)	300 dia. Pipe (m ³)	ST.5 (m ³)	ADS UNIT (m ³)	ST.4 (m ³)	300 dia. Pipe (m ³)	ST.3 (m ³)	300 dia. Pipe (m ³)	ST.2 (m ³)	300 dia. Pipe (m ³)	Surface (m ³)		Total (m ³)	
268.818	0.00	0.000	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	BOTTOM OF STONE
268.843	0.00	0.130	0.00	3.455	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	3.59	
268.868	0.00	0.170	0.00	6.911	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	7.08	
268.894	0.00	0.210	0.00	10.366	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	10.58	
268.919	0.00	0.250	0.00	13.821	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	14.07	
268.945	0.00	0.290	0.00	17.276	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	17.57	
268.970	0.00	0.320	0.00	20.732	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	21.05	BOTTOM OF CHAMBER
268.995	0.029	0.350	0.00	27.358	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	27.74	
269.021	0.057	0.370	0.00	33.919	0.00	0.004	0.00	0.00	0.00	0.00	0.00	0.00	34.35	
269.046	0.086	0.390	0.00	40.391	0.00	0.030	0.00	0.011	0.00	0.00	0.00	0.00	40.91	
269.072	0.115	0.390	0.00	46.775	0.00	0.085	0.00	0.024	0.00	0.00	0.00	0.00	47.39	
269.097	0.144	0.390	0.00	53.048	0.00	0.150	0.00	0.038	0.00	0.005	0.00	0.00	53.77	
269.122	0.172	0.390	0.00	59.189	0.00	0.230	0.00	0.053	0.00	0.023	0.00	0.00	60.06	5 YR. STORM ELEV = 269.110
269.148	0.201	0.390	0.00	65.195	0.00	0.320	0.00	0.069	0.00	0.063	0.00	0.00	66.24	
269.173	0.230	0.390	0.00	71.070	0.00	0.400	0.00	0.086	0.00	0.120	0.00	0.00	72.30	
269.199	0.259	0.390	0.000	76.769	0.00	0.490	0.00	0.100	0.00	0.210	0.00	0.00	78.22	
269.224	0.287	0.390	0.000	82.268	0.00	0.580	0.00	0.120	0.00	0.320	0.00	0.00	83.97	
269.249	0.316	0.390	0.009	87.546	0.00	0.650	0.00	0.130	0.00	0.450	0.00	0.00	89.49	
269.275	0.345	0.390	0.020	92.557	0.00	0.730	0.00	0.140	0.00	0.620	0.00	0.00	94.80	
269.300	0.373	0.390	0.031	97.217	0.00	0.780	0.00	0.150	0.00	0.800	0.00	0.00	99.74	
269.326	0.402	0.390	0.042	101.260	0.011	0.810	0.011	0.150	0.00	1.010	0.00	0.00	104.09	
269.351	0.431	0.390	0.053	105.057	0.023	0.820	0.023	0.150	0.00	1.210	0.00	0.00	108.16	
269.376	0.460	0.390	0.065	108.642	0.034	0.820	0.034	0.150	0.000	1.400	0.00	0.00	111.99	TOP OF CHAMBER
269.402	0.488	0.390	0.076	112.097	0.045	0.820	0.045	0.150	0.002	1.580	0.00	0.00	115.69	
269.427	0.517	0.390	0.087	115.553	0.056	0.820	0.056	0.150	0.031	1.730	0.00	0.00	119.39	
269.453	0.546	0.390	0.098	119.008	0.067	0.820	0.067	0.150	0.059	1.860	0.00	0.00	123.07	
269.478	0.575	0.390	0.110	122.463	0.079	0.820	0.079	0.150	0.088	1.950	0.00	0.00	126.70	
269.503	0.603	0.390	0.121	125.919	0.090	0.820	0.090	0.150	0.117	2.030	0.00	0.00	130.33	
269.529	0.632	0.390	0.132	129.374	0.101	0.820	0.101	0.150	0.146	2.080	0.00	0.00	133.93	TOP OF STONE
269.554	0.661	0.390	0.143	129.374	0.112	0.820	0.112	0.150	0.174	2.110	0.00	0.00	134.05	
269.580	0.689	0.390	0.154	129.374	0.124	0.820	0.124	0.150	0.203	2.120	0.00	0.00	134.15	
269.605	0.718	0.390	0.166	129.374	0.135	0.820	0.135	0.150	0.232	2.120	0.00	0.00	134.24	
269.630	0.747	0.390	0.177	129.374	0.146	0.820	0.146	0.150	0.261	2.120	0.00	0.00	134.33	
269.656	0.776	0.390	0.188	129.374	0.157	0.820	0.157	0.150	0.289	2.120	0.00	0.00	134.42	
269.681	0.804	0.390	0.199	129.374	0.168	0.820	0.168	0.150	0.318	2.120	0.00	0.00	134.51	
269.707	0.833	0.390	0.211	129.374	0.180	0.820	0.180	0.150	0.347	2.120	0.00	0.00	134.60	
269.732	0.862	0.390	0.222	129.374	0.191	0.820	0.191	0.150	0.375	2.120	0.00	0.00	134.69	
269.757	0.891	0.390	0.233	129.374	0.202	0.820	0.202	0.150	0.404	2.120	0.00	0.00	134.79	
269.783	0.919	0.390	0.244	129.374	0.213	0.820	0.213	0.150	0.433	2.120	0.00	0.00	134.88	
269.808	0.948	0.390	0.255	129.374	0.225	0.820	0.225	0.150	0.462	2.120	0.00	0.00	134.97	
269.834	0.977	0.390	0.267	129.374	0.236	0.820	0.236	0.150	0.490	2.120	0.00	0.00	135.06	
269.859	1.005	0.390	0.278	129.374	0.247	0.820	0.247	0.150	0.519	2.120	0.010	0.00	135.16	
269.884	1.034	0.390	0.289	129.374	0.258	0.820	0.258	0.150	0.548	2.120	0.160	0.00	135.40	
269.910	1.063	0.390	0.300	129.374	0.269	0.820	0.269	0.150	0.577	2.120	0.720	0.00	136.05	
269.935	1.092	0.390	0.312	129.374	0.281	0.820	0.281	0.150	0.605	2.120	1.870	0.00	137.29	100 YR. STORM ELEV = 269.939
269.961	1.120	0.390	0.323	129.374	0.292	0.820	0.292	0.150	0.634	2.120	3.960	0.00	139.47	

VOLUME CALCULATION DOES NOT INCLUDE CATCHBASIN LEEDS AND CATCHBASIN STORAGE

PROJECT INFORMATION	
ENGINEERED PRODUCT MANAGER:	VIVEK SHARMA 647-463-9803 VIVEK.SHARMA@ADS-PIPE.COM
ADS SALES REP:	ANDREW OKOLISAN 519-670-0564 ANDREW.OKOLISAN@ADS-PIPE.COM
PROJECT NO:	S080669



ADVANCED DRAINAGE SYSTEMS, INC.



HURON MOTOR PRODUCTS

EXTER, ONTARIO -CANADA

STORMTECH CHAMBER SPECIFICATIONS

1. CHAMBERS SHALL BE STORMTECH SC-740 OR SC-310.
2. CHAMBERS SHALL BE MANUFACTURED FROM VIRGIN POLYPROPYLENE OR POLYETHYLENE RESINS.
3. CHAMBER ROWS SHALL PROVIDE CONTINUOUS, UNOBSTRUCTED INTERNAL SPACE WITH NO INTERNAL SUPPORT PANELS THAT WOULD IMPEDE FLOW OR LIMIT ACCESS FOR INSPECTION.
4. THE STRUCTURAL DESIGN OF THE CHAMBERS, THE STRUCTURAL BACKFILL, AND THE INSTALLATION REQUIREMENTS SHALL ENSURE THAT THE LOAD FACTORS SPECIFIED IN THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, SECTION 12.12, ARE MET FOR: 1) LONG-DURATION DEAD LOADS AND 2) SHORT-DURATION LIVE LOADS, BASED ON THE CSA S6 CL-625 TRUCK AND THE AASHTO DESIGN TRUCK WITH CONSIDERATION FOR IMPACT AND MULTIPLE VEHICLE PRESENCES.
5. CHAMBERS SHALL BE CERTIFIED TO CSA B184, "POLYMERIC SUB-SURFACE STORMWATER MANAGEMENT STRUTURES", AND MEET ASTM F2922 (POLYETHYLENE) OR ASTM F2418-16 (POLYPROPYLENE), "STANDARD SPECIFICATION FOR THERMOPLASTIC CORRUGATED WALL STORMWATER COLLECTION CHAMBERS".
6. CHAMBERS SHALL BE DESIGNED AND ALLOWABLE LOADS DETERMINED IN ACCORDANCE WITH ASTM F2787, "STANDARD PRACTICE FOR STRUCTURAL DESIGN OF THERMOPLASTIC CORRUGATED WALL STORMWATER COLLECTION CHAMBERS".
7. ONLY CHAMBERS THAT ARE APPROVED BY THE SITE DESIGN ENGINEER WILL BE ALLOWED. THE CHAMBER MANUFACTURER SHALL SUBMIT THE FOLLOWING UPON REQUEST TO THE SITE DESIGN ENGINEER FOR APPROVAL BEFORE DELIVERING CHAMBERS TO THE PROJECT SITE:
 - a. A STRUCTURAL EVALUATION SEALED BY A REGISTERED PROFESSIONAL ENGINEER THAT DEMONSTRATES THAT THE SAFETY FACTORS ARE GREATER THAN OR EQUAL TO 1.95 FOR DEAD LOAD AND 1.75 FOR LIVE LOAD, THE MINIMUM REQUIRED BY ASTM F2787 AND BY AASHTO FOR THERMOPLASTIC PIPE.
 - b. A STRUCTURAL EVALUATION SEALED BY A REGISTERED PROFESSIONAL ENGINEER THAT DEMONSTRATES THAT THE LOAD FACTORS SPECIFIED IN THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, SECTION 12.12, ARE MET. THE 50 YEAR CREEP MODULUS DATA SPECIFIED IN ASTM F2418 OR ASTM F2922 MUST BE USED AS PART OF THE AASHTO STRUCTURAL EVALUATION TO VERIFY LONG-TERM PERFORMANCE.
 - c. STRUCTURAL CROSS SECTION DETAIL ON WHICH THE STRUCTURAL EVALUATION IS BASED.
8. CHAMBERS AND END CAPS SHALL BE PRODUCED AT AN ISO 9001 CERTIFIED MANUFACTURING FACILITY.

IMPORTANT - NOTES FOR THE BIDDING AND INSTALLATION OF THE SC-310/SC-740 SYSTEM

1. STORMTECH SC-310 & SC-740 CHAMBERS SHALL NOT BE INSTALLED UNTIL THE MANUFACTURER'S REPRESENTATIVE HAS COMPLETED A PRE-CONSTRUCTION MEETING WITH THE INSTALLERS.
2. STORMTECH SC-310 & SC-740 CHAMBERS SHALL BE INSTALLED IN ACCORDANCE WITH THE "STORMTECH SC-310/SC-740/SC-780 CONSTRUCTION GUIDE".
3. CHAMBERS ARE NOT TO BE BACKFILLED WITH A DOZER OR AN EXCAVATOR SITUATED OVER THE CHAMBERS. STORMTECH RECOMMENDS 3 BACKFILL METHODS:
 - STONESHOOTER LOCATED OFF THE CHAMBER BED.
 - BACKFILL AS ROWS ARE BUILT USING AN EXCAVATOR ON THE FOUNDATION STONE OR SUBGRADE.
 - BACKFILL FROM OUTSIDE THE EXCAVATION USING A LONG BOOM HOE OR EXCAVATOR.
4. THE FOUNDATION STONE SHALL BE LEVELED AND COMPACTED PRIOR TO PLACING CHAMBERS.
5. JOINTS BETWEEN CHAMBERS SHALL BE PROPERLY SEATED PRIOR TO PLACING STONE.
6. MAINTAIN MINIMUM - 150 mm (6") SPACING BETWEEN THE CHAMBER ROWS.
7. EMBEDMENT STONE SURROUNDING CHAMBERS MUST BE A CLEAN, CRUSHED, ANGULAR STONE 20-50 mm (3/4-2").
8. THE CONTRACTOR MUST REPORT ANY KNOWN DISCREPANCIES WITH CHAMBER FOUNDATION MATERIALS BEARING CAPACITIES TO THE SITE DESIGN ENGINEER.
9. ADS RECOMMENDS THE USE OF "FLEXSTORM CATCH IT" INSERTS DURING CONSTRUCTION FOR ALL INLETS TO PROTECT THE SUBSURFACE STORMWATER MANAGEMENT SYSTEM FROM CONSTRUCTION SITE RUNOFF.

NOTES FOR CONSTRUCTION EQUIPMENT

1. STORMTECH SC-310 & SC-740 CHAMBERS SHALL BE INSTALLED IN ACCORDANCE WITH THE "STORMTECH SC-310/SC-740/DC-780 CONSTRUCTION GUIDE".
2. THE USE OF CONSTRUCTION EQUIPMENT OVER SC-310 & SC-740 CHAMBERS IS LIMITED:
 - NO EQUIPMENT IS ALLOWED ON BARE CHAMBERS.
 - NO RUBBER TIRE LOADERS, DUMP TRUCKS, OR EXCAVATORS ARE ALLOWED UNTIL PROPER FILL DEPTHS ARE REACHED IN ACCORDANCE WITH THE "STORMTECH SC-310/SC-740/DC-780 CONSTRUCTION GUIDE".
 - WEIGHT LIMITS FOR CONSTRUCTION EQUIPMENT CAN BE FOUND IN THE "STORMTECH SC-310/SC-740/DC-780 CONSTRUCTION GUIDE".
3. FULL 900 mm (36") OF STABILIZED COVER MATERIALS OVER THE CHAMBERS IS REQUIRED FOR DUMP TRUCK TRAVEL OR DUMPING.

USE OF A DOZER TO PUSH EMBEDMENT STONE BETWEEN THE ROWS OF CHAMBERS MAY CAUSE DAMAGE TO THE CHAMBERS AND IS NOT AN ACCEPTABLE BACKFILL METHOD. ANY CHAMBERS DAMAGED BY THE "DUMP AND PUSH" METHOD ARE NOT COVERED UNDER THE STORMTECH STANDARD WARRANTY.

CONTACT STORMTECH AT 1-888-892-2694 WITH ANY QUESTIONS ON INSTALLATION REQUIREMENTS OR WEIGHT LIMITS FOR CONSTRUCTION EQUIPMENT.

PROPOSED LAYOUT

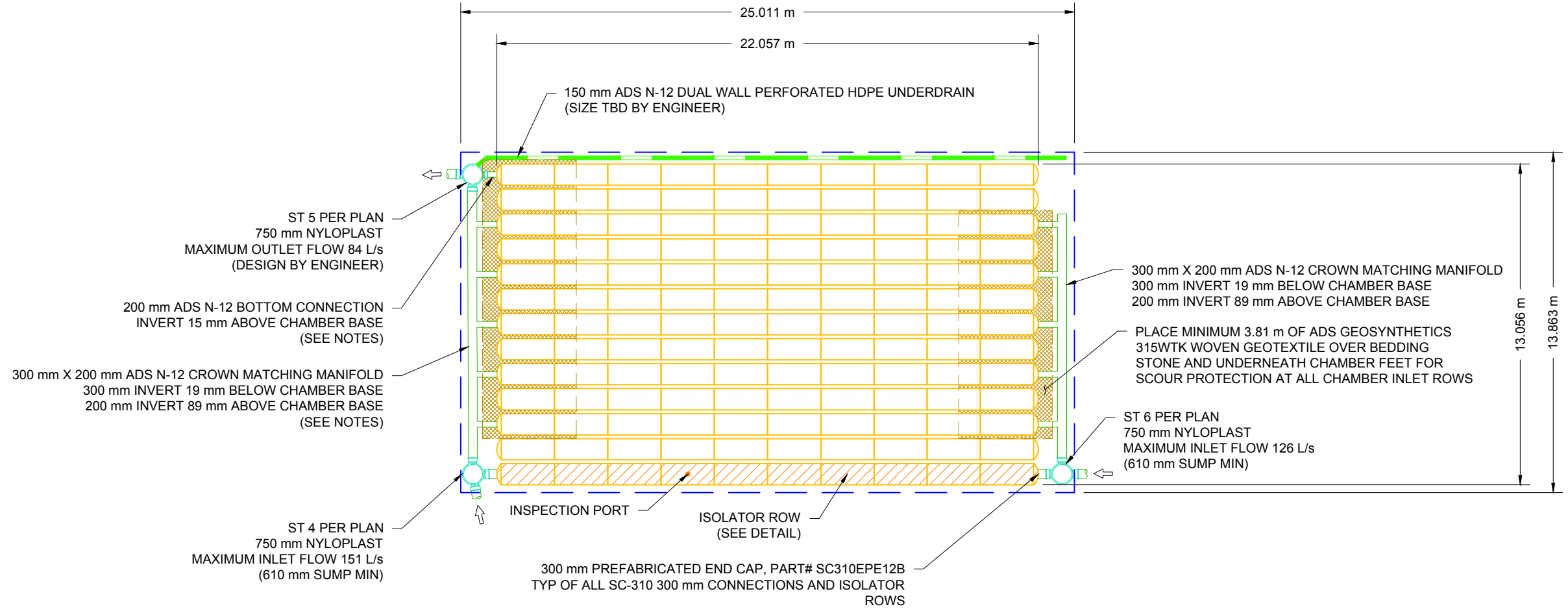
130	STORMTECH SC-310 CHAMBERS
26	STORMTECH SC-310 END CAPS
152	STONE ABOVE (mm)
152	STONE BELOW (mm)
40	% STONE VOID
131.2	INSTALLED SYSTEM VOLUME (m³) (PERIMETER STONE INCLUDED)
347	SYSTEM AREA (m²)
78	SYSTEM PERIMETER (m)

PROPOSED ELEVATIONS

MAXIMUM ALLOWABLE GRADE (TOP OF PAVEMENT/UNPAVED):	271.814
MINIMUM ALLOWABLE GRADE (UNPAVED WITH TRAFFIC):	269.986
MINIMUM ALLOWABLE GRADE (UNPAVED NO TRAFFIC):	269.833
MINIMUM ALLOWABLE GRADE (BASE OF FLEXIBLE PAVEMENT):	269.833
MINIMUM ALLOWABLE GRADE (TOP OF RIGID PAVEMENT):	269.833
TOP OF STONE:	269.528
TOP OF SC-310 CHAMBER:	269.376
300 mm X 200 mm MANIFOLD INVERT (200 mm PIPE):	269.059
300 mm ISOLATOR ROW INVERT:	268.994
200 mm BOTTOM CONNECTION INVERT :	268.985
BOTTOM OF SC-310 CHAMBER:	268.970
300 mm X 200 mm MANIFOLD INVERT (300 mm PIPE):	268.951
UNDERDRAIN INVERT:	268.818
BOTTOM OF STONE:	268.818

NOTES

- MANIFOLD SIZE TO BE DETERMINED BY SITE DESIGN ENGINEER. SEE TECH SHEET #7 FOR MANIFOLD SIZING GUIDANCE.
- DUE TO THE ADAPTATION OF THIS CHAMBER SYSTEM TO SPECIFIC SITE AND DESIGN CONSTRAINTS, IT MAY BE NECESSARY TO CUT AND COUPLE ADDITIONAL PIPE TO STANDARD MANIFOLD COMPONENTS IN THE FIELD.
- THE SITE DESIGN ENGINEER MUST REVIEW ELEVATIONS AND IF NECESSARY ADJUST GRADING TO ENSURE THE CHAMBER COVER REQUIREMENTS ARE MET.
- THIS CHAMBER SYSTEM WAS DESIGNED WITHOUT SITE-SPECIFIC INFORMATION ON SOIL CONDITIONS OR BEARING CAPACITY. THE SITE DESIGN ENGINEER IS RESPONSIBLE FOR DETERMINING THE SUITABILITY OF THE SOIL AND PROVIDING THE BEARING CAPACITY OF THE INSITU SOILS. THE BASE STONE DEPTH MAY BE INCREASED OR DECREASED ONCE THIS INFORMATION IS PROVIDED.



HURON MOTOR PRODUCTS	
EXTER, ONTARIO -CANADA	
DATE: 4/24/2018	DRAWN: PM
PROJECT #: S080669	CHECKED: CJD

REV	DWN	CKD	DESCRIPTION
4/25/2018	PM	RWD	UPDATE PER ENG. REQUEST
07-13-18	JKL	GF	UPDATED PER ENG. REQUEST

StormTech
Determining Tomorrow's Water Quality
 70 INWOOD ROAD, SUITE 3 | ROCKY HILL, CT | 06867
 860-528-8188 | 888-892-2894 | WWW.STORMTECH.COM

ADS
 ADVANCED DRAINAGE SYSTEMS, INC.
 4640 TRUEMAN BLVD
 HILLIARD, OH 43026

SCALE = 1 : 200

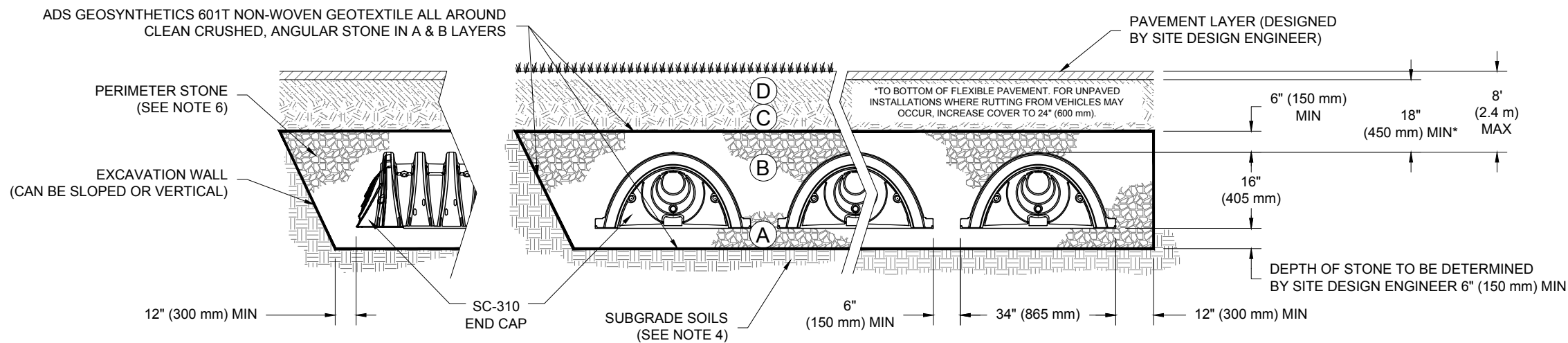
THIS DRAWING HAS BEEN PREPARED BASED ON INFORMATION PROVIDED TO ADS UNDER THE DIRECTION OF THE SITE DESIGN ENGINEER OR OTHER PROJECT REPRESENTATIVE. THE SITE DESIGN ENGINEER SHALL REVIEW THIS DRAWING PRIOR TO CONSTRUCTION. IT IS THE ULTIMATE RESPONSIBILITY OF THE SITE DESIGN ENGINEER TO ENSURE THAT THE PRODUCT(S) DEPICTED AND ALL ASSOCIATED DETAILS MEET ALL APPLICABLE LAWS, REGULATIONS, AND PROJECT REQUIREMENTS.

ACCEPTABLE FILL MATERIALS: STORMTECH SC-310 CHAMBER SYSTEMS

MATERIAL LOCATION	DESCRIPTION	AASHTO MATERIAL CLASSIFICATIONS	COMPACTION / DENSITY REQUIREMENT
D	FINAL FILL: FILL MATERIAL FOR LAYER 'D' STARTS FROM THE TOP OF THE 'C' LAYER TO THE BOTTOM OF FLEXIBLE PAVEMENT OR UNPAVED FINISHED GRADE ABOVE. NOTE THAT PAVEMENT SUBBASE MAY BE PART OF THE 'D' LAYER	N/A	PREPARE PER SITE DESIGN ENGINEER'S PLANS. PAVED INSTALLATIONS MAY HAVE STRINGENT MATERIAL AND PREPARATION REQUIREMENTS.
C	INITIAL FILL: FILL MATERIAL FOR LAYER 'C' STARTS FROM THE TOP OF THE EMBEDMENT STONE ('B' LAYER) TO 18" (450 mm) ABOVE THE TOP OF THE CHAMBER. NOTE THAT PAVEMENT SUBBASE MAY BE A PART OF THE 'C' LAYER.	AASHTO M145 ¹ A-1, A-2-4, A-3 OR AASHTO M43 ¹ 3, 357, 4, 467, 5, 56, 57, 6, 67, 68, 7, 78, 8, 89, 9, 10	BEGIN COMPACTIONS AFTER 12" (300 mm) OF MATERIAL OVER THE CHAMBERS IS REACHED. COMPACT ADDITIONAL LAYERS IN 6" (150 mm) MAX LIFTS TO A MIN. 95% PROCTOR DENSITY FOR WELL GRADED MATERIAL AND 95% RELATIVE DENSITY FOR PROCESSED AGGREGATE MATERIALS. ROLLER GROSS VEHICLE WEIGHT NOT TO EXCEED 12,000 lbs (53 kN). DYNAMIC FORCE NOT TO EXCEED 20,000 lbs (89 kN).
B	EMBEDMENT STONE: FILL SURROUNDING THE CHAMBERS FROM THE FOUNDATION STONE ('A' LAYER) TO THE 'C' LAYER ABOVE.	AASHTO M43 ¹ 3, 357, 4, 467, 5, 56, 57	NO COMPACTION REQUIRED.
A	FOUNDATION STONE: FILL BELOW CHAMBERS FROM THE SUBGRADE UP TO THE FOOT (BOTTOM) OF THE CHAMBER.	AASHTO M43 ¹ 3, 357, 4, 467, 5, 56, 57	PLATE COMPACT OR ROLL TO ACHIEVE A FLAT SURFACE. ^{2,3}

PLEASE NOTE:

- THE LISTED AASHTO DESIGNATIONS ARE FOR GRADATIONS ONLY. THE STONE MUST ALSO BE CLEAN, CRUSHED, ANGULAR. FOR EXAMPLE, A SPECIFICATION FOR #4 STONE WOULD STATE: "CLEAN, CRUSHED, ANGULAR NO. 4 (AASHTO M43) STONE".
- STORMTECH COMPACTION REQUIREMENTS ARE MET FOR 'A' LOCATION MATERIALS WHEN PLACED AND COMPACTED IN 6" (150 mm) (MAX) LIFTS USING TWO FULL COVERAGES WITH A VIBRATORY COMPACTOR.
- WHERE INFILTRATION SURFACES MAY BE COMPROMISED BY COMPACTION, FOR STANDARD DESIGN LOAD CONDITIONS, A FLAT SURFACE MAY BE ACHIEVED BY RAKING OR DRAGGING WITHOUT COMPACTION EQUIPMENT. FOR SPECIAL LOAD DESIGNS, CONTACT STORMTECH FOR COMPACTION REQUIREMENTS.



NOTES:

- SC-310 CHAMBERS SHALL CONFORM TO THE REQUIREMENTS OF ASTM F2418 "STANDARD SPECIFICATION FOR POLYPROPYLENE (PP) CORRUGATED WALL STORMWATER COLLECTION CHAMBERS", OR ASTM F2922 "STANDARD SPECIFICATION FOR POLYETHYLENE (PE) CORRUGATED WALL STORMWATER COLLECTION CHAMBERS".
- SC-310 CHAMBERS SHALL BE DESIGNED IN ACCORDANCE WITH ASTM F2787 "STANDARD PRACTICE FOR STRUCTURAL DESIGN OF THERMOPLASTIC CORRUGATED WALL STORMWATER COLLECTION CHAMBERS".
- "ACCEPTABLE FILL MATERIALS" TABLE ABOVE PROVIDES MATERIAL LOCATIONS, DESCRIPTIONS, GRADATIONS, AND COMPACTION REQUIREMENTS FOR FOUNDATION, EMBEDMENT, AND FILL MATERIALS.
- THE SITE DESIGN ENGINEER IS RESPONSIBLE FOR ASSESSING THE BEARING RESISTANCE (ALLOWABLE BEARING CAPACITY) OF THE SUBGRADE SOILS AND THE DEPTH OF FOUNDATION STONE WITH CONSIDERATION FOR THE RANGE OF EXPECTED SOIL MOISTURE CONDITIONS.
- PERIMETER STONE MUST BE EXTENDED HORIZONTALLY TO THE EXCAVATION WALL FOR BOTH VERTICAL AND SLOPED EXCAVATION WALLS.
- ONCE LAYER 'C' IS PLACED, ANY SOIL/MATERIAL CAN BE PLACED IN LAYER 'D' UP TO THE FINISHED GRADE. MOST PAVEMENT SUBBASE SOILS CAN BE USED TO REPLACE THE MATERIAL REQUIREMENTS OF LAYER 'C' OR 'D' AT THE SITE DESIGN ENGINEER'S DISCRETION.

HURON MOTOR PRODUCTS		EXTER, ONTARIO - CANADA	
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PROJECT #:	S080669	CHECKED:	CJD
REV	DWN	CKD	DESCRIPTION
4/25/2018	PM	RWD	UPDATE PER ENG
07-13-18	JKL	GFI	UPDATED PER ENG. REQUEST

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Dedicated. Reimagine. Water. Quality.

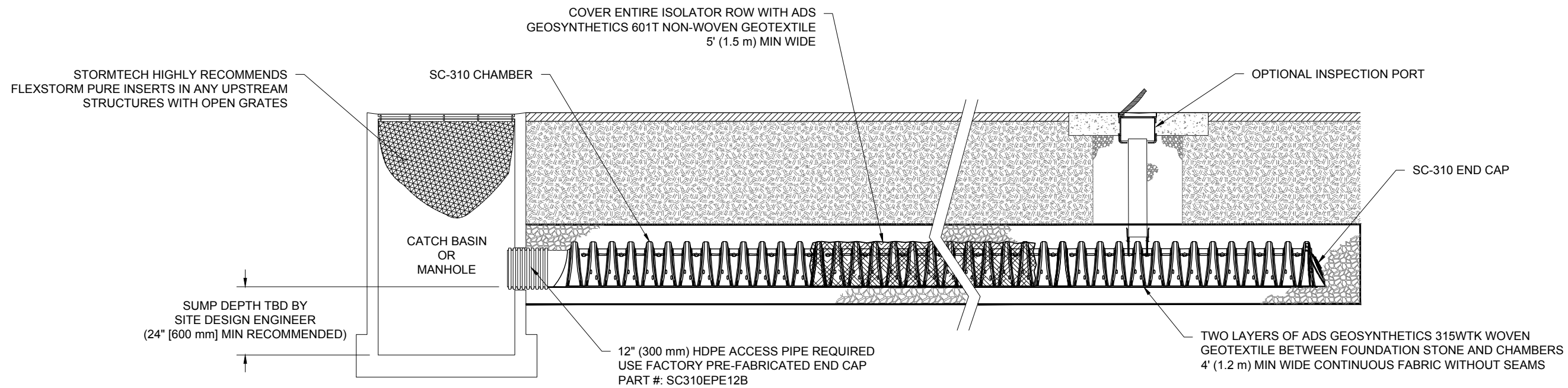
70 INWOOD ROAD, SUITE 3 | ROCKY HILL | CT | 06067
860-528-8188 | 888-892-2894 | WWW.STORMTECH.COM

4640 TRUEMAN BLVD
HILLIARD, OH 43026

ADVANCED DRAINAGE SYSTEMS, INC.

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3 SHEET
OF 6



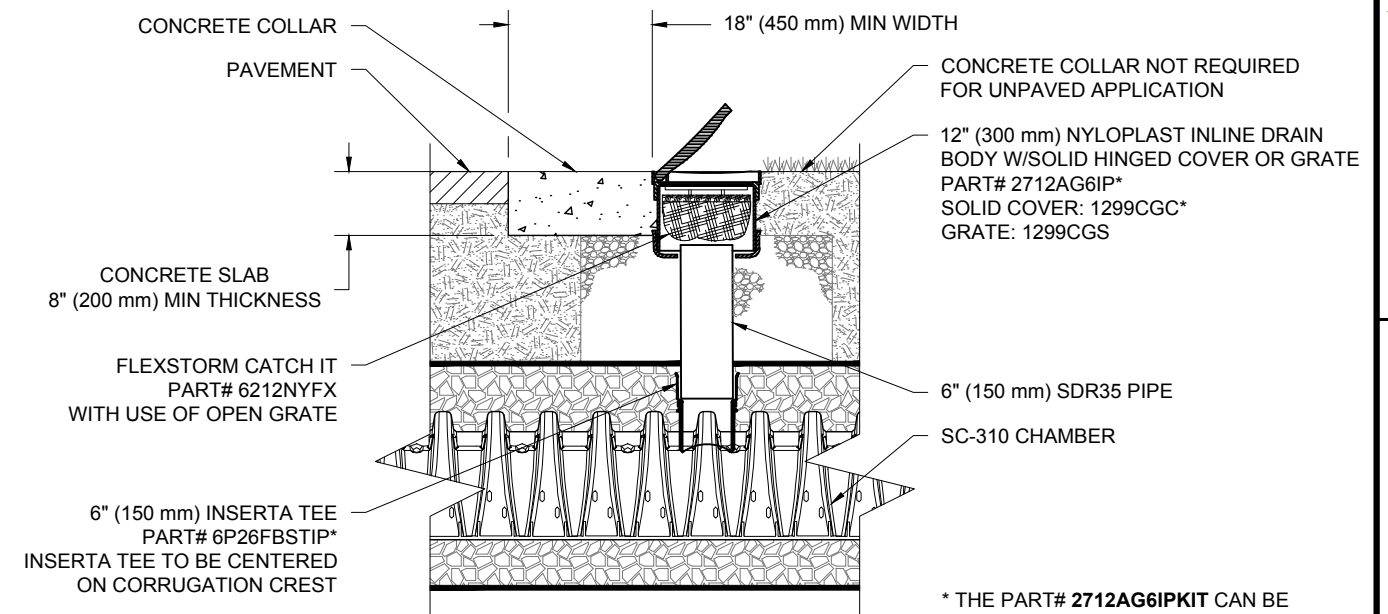
SC-310 ISOLATOR ROW DETAIL
NTS

INSPECTION & MAINTENANCE

- STEP 1) INSPECT ISOLATOR ROW FOR SEDIMENT
- A. INSPECTION PORTS (IF PRESENT)
 - A.1. REMOVE/OPEN LID ON NYLOPLAST INLINE DRAIN
 - A.2. REMOVE AND CLEAN FLEXSTORM FILTER IF INSTALLED
 - A.3. USING A FLASHLIGHT AND STADIA ROD, MEASURE DEPTH OF SEDIMENT AND RECORD ON MAINTENANCE LOG
 - A.4. LOWER A CAMERA INTO ISOLATOR ROW FOR VISUAL INSPECTION OF SEDIMENT LEVELS (OPTIONAL)
 - A.5. IF SEDIMENT IS AT, OR ABOVE, 3" (80 mm) PROCEED TO STEP 2. IF NOT, PROCEED TO STEP 3.
 - B. ALL ISOLATOR ROWS
 - B.1. REMOVE COVER FROM STRUCTURE AT UPSTREAM END OF ISOLATOR ROW
 - B.2. USING A FLASHLIGHT, INSPECT DOWN THE ISOLATOR ROW THROUGH OUTLET PIPE
 - i) MIRRORS ON POLES OR CAMERAS MAY BE USED TO AVOID A CONFINED SPACE ENTRY
 - ii) FOLLOW OSHA REGULATIONS FOR CONFINED SPACE ENTRY IF ENTERING MANHOLE
 - B.3. IF SEDIMENT IS AT, OR ABOVE, 3" (80 mm) PROCEED TO STEP 2. IF NOT, PROCEED TO STEP 3.
- STEP 2) CLEAN OUT ISOLATOR ROW USING THE JETVAC PROCESS
- A. A FIXED CULVERT CLEANING NOZZLE WITH REAR FACING SPREAD OF 45" (1.1 m) OR MORE IS PREFERRED
 - B. APPLY MULTIPLE PASSES OF JETVAC UNTIL BACKFLUSH WATER IS CLEAN
 - C. VACUUM STRUCTURE SUMP AS REQUIRED
- STEP 3) REPLACE ALL COVERS, GRATES, FILTERS, AND LIDS; RECORD OBSERVATIONS AND ACTIONS.
- STEP 4) INSPECT AND CLEAN BASINS AND MANHOLES UPSTREAM OF THE STORMTECH SYSTEM.

NOTES

1. INSPECT EVERY 6 MONTHS DURING THE FIRST YEAR OF OPERATION. ADJUST THE INSPECTION INTERVAL BASED ON PREVIOUS OBSERVATIONS OF SEDIMENT ACCUMULATION AND HIGH WATER ELEVATIONS.
2. CONDUCT JETTING AND VACTORING ANNUALLY OR WHEN INSPECTION SHOWS THAT MAINTENANCE IS NECESSARY.



SC-310 6" INSPECTION PORT DETAIL
NTS

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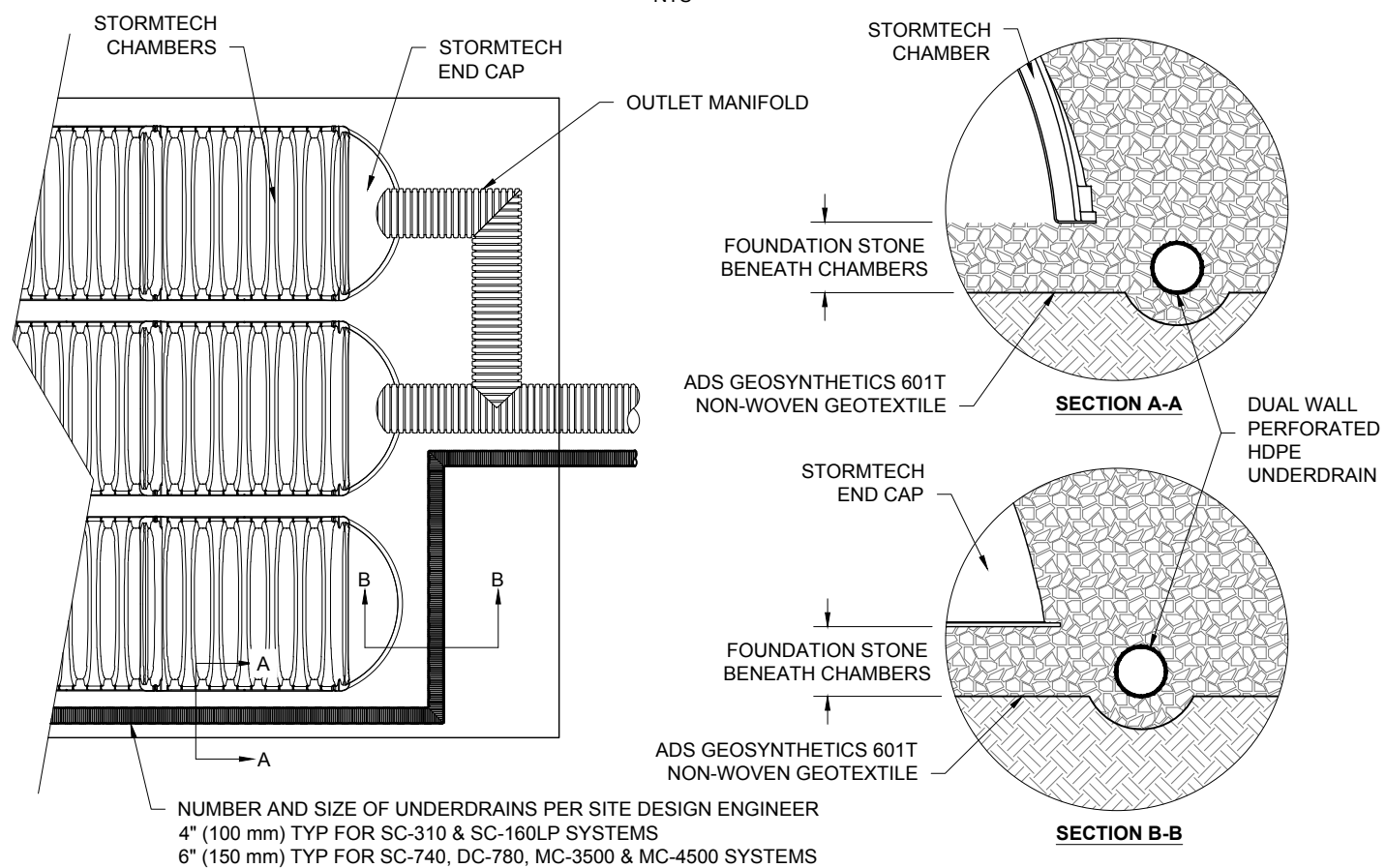
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OF 6

UNDERDRAIN DETAIL

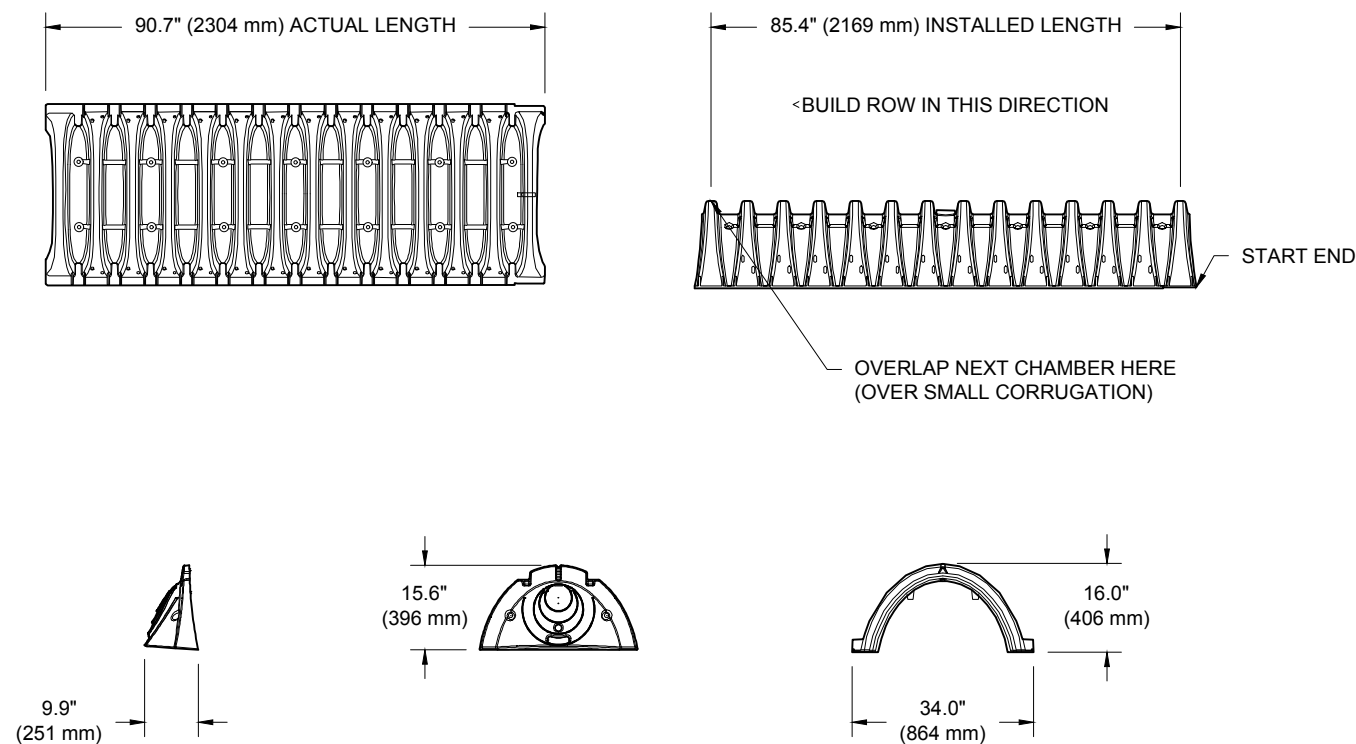
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NUMBER AND SIZE OF UNDERDRAINS PER SITE DESIGN ENGINEER
 4" (100 mm) TYP FOR SC-310 & SC-160LP SYSTEMS
 6" (150 mm) TYP FOR SC-740, DC-780, MC-3500 & MC-4500 SYSTEMS

SC-310 TECHNICAL SPECIFICATION

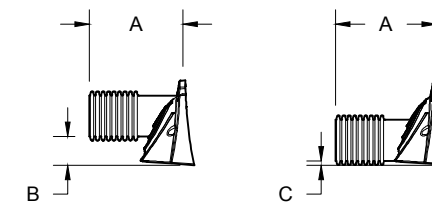
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NOMINAL CHAMBER SPECIFICATIONS

SIZE (W X H X INSTALLED LENGTH)	34.0" X 16.0" X 85.4"	(864 mm X 406 mm X 2169 mm)
CHAMBER STORAGE	14.7 CUBIC FEET	(0.42 m ³)
MINIMUM INSTALLED STORAGE*	31.0 CUBIC FEET	(0.88 m ³)
WEIGHT	35.0 lbs.	(16.8 kg)

*ASSUMES 6" (152 mm) ABOVE, BELOW, AND BETWEEN CHAMBERS



PRE-FAB STUBS AT BOTTOM OF END CAP FOR PART NUMBERS ENDING WITH "B"
 PRE-FAB STUBS AT TOP OF END CAP FOR PART NUMBERS ENDING WITH "T"
 PRE CORED END CAPS END WITH "PC"

PART #	STUB	A	B	C
SC310EPE06T / SC310EPE06TPC	6" (150 mm)	9.6" (244 mm)	5.8" (147 mm)	---
SC310EPE06B / SC310EPE06BPC			---	0.5" (13 mm)
SC310EPE08T / SC310EPE08TPC	8" (200 mm)	11.9" (302 mm)	3.5" (89 mm)	---
SC310EPE08B / SC310EPE08BPC			---	0.6" (15 mm)
SC310EPE10T / SC310EPE10TPC	10" (250 mm)	12.7" (323 mm)	1.4" (36 mm)	---
SC310EPE10B / SC310EPE10BPC			---	0.7" (18 mm)
SC310EPE12B	12" (300 mm)	13.5" (343 mm)	---	0.9" (23 mm)

ALL STUBS, EXCEPT FOR THE SC310EPE12B ARE PLACED AT BOTTOM OF END CAP SUCH THAT THE OUTSIDE DIAMETER OF THE STUB IS FLUSH WITH THE BOTTOM OF THE END CAP. FOR ADDITIONAL INFORMATION CONTACT STORMTECH AT 1-888-892-2694.

* FOR THE SC310EPE12B THE 12" (300 mm) STUB LIES BELOW THE BOTTOM OF THE END CAP APPROXIMATELY 0.25" (6 mm). BACKFILL MATERIAL SHOULD BE REMOVED FROM BELOW THE N-12 STUB SO THAT THE FITTING SITS LEVEL.

NOTE: ALL DIMENSIONS ARE NOMINAL

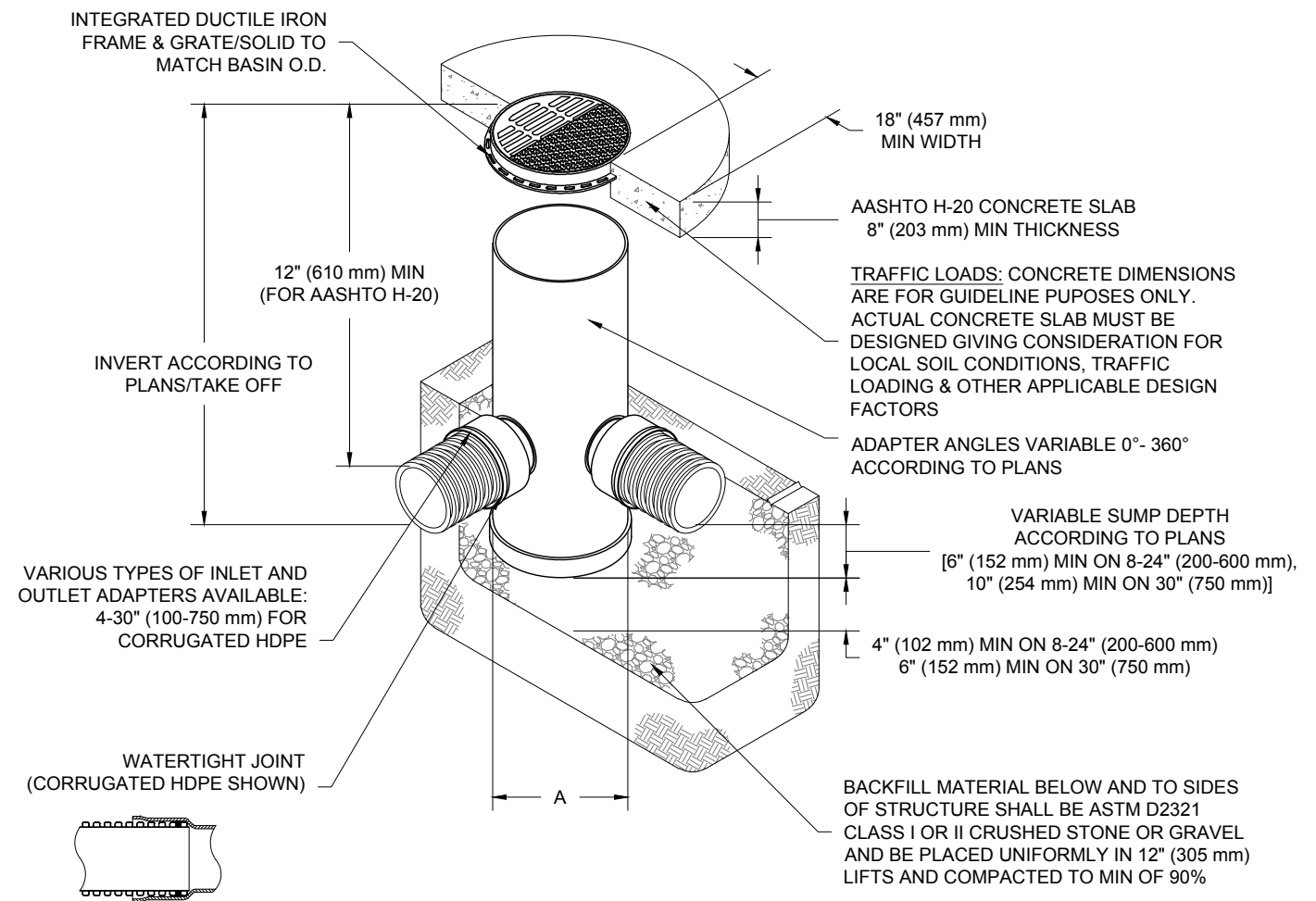
HURON MOTOR PRODUCTS
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NYLOPLAST DRAIN BASIN

NTS



TRAFFIC LOADS: CONCRETE DIMENSIONS ARE FOR GUIDELINE PURPOSES ONLY. ACTUAL CONCRETE SLAB MUST BE DESIGNED GIVING CONSIDERATION FOR LOCAL SOIL CONDITIONS, TRAFFIC LOADING & OTHER APPLICABLE DESIGN FACTORS

ADAPTER ANGLES VARIABLE 0° - 360° ACCORDING TO PLANS

VARIABLE SUMP DEPTH ACCORDING TO PLANS
 [6" (152 mm) MIN ON 8-24" (200-600 mm),
 10" (254 mm) MIN ON 30" (750 mm)]

4" (102 mm) MIN ON 8-24" (200-600 mm)
 6" (152 mm) MIN ON 30" (750 mm)

BACKFILL MATERIAL BELOW AND TO SIDES OF STRUCTURE SHALL BE ASTM D2321 CLASS I OR II CRUSHED STONE OR GRAVEL AND BE PLACED UNIFORMLY IN 12" (305 mm) LIFTS AND COMPACTED TO MIN OF 90%

NOTES

- 8-30" (200-750 mm) GRATES/SOLID COVERS SHALL BE DUCTILE IRON PER ASTM A536 GRADE 70-50-05
- 12-30" (300-750 mm) FRAMES SHALL BE DUCTILE IRON PER ASTM A536 GRADE 70-50-05
- DRAIN BASIN TO BE CUSTOM MANUFACTURED ACCORDING TO PLAN DETAILS
- DRAINAGE CONNECTION STUB JOINT TIGHTNESS SHALL CONFORM TO ASTM D3212 FOR CORRUGATED HDPE (ADS & HANCOR DUAL WALL) & SDR 35 PVC
- FOR COMPLETE DESIGN AND PRODUCT INFORMATION: WWW.NYLOPLAST-US.COM
- TO ORDER CALL: 800-821-6710

A	PART #	GRATE/SOLID COVER OPTIONS		
8" (200 mm)	2808AG	PEDESTRIAN LIGHT DUTY	STANDARD LIGHT DUTY	SOLID LIGHT DUTY
10" (250 mm)	2810AG	PEDESTRIAN LIGHT DUTY	STANDARD LIGHT DUTY	SOLID LIGHT DUTY
12" (300 mm)	2812AG	PEDESTRIAN AASHTO H-10	STANDARD AASHTO H-20	SOLID AASHTO H-20
15" (375 mm)	2815AG	PEDESTRIAN AASHTO H-10	STANDARD AASHTO H-20	SOLID AASHTO H-20
18" (450 mm)	2818AG	PEDESTRIAN AASHTO H-10	STANDARD AASHTO H-20	SOLID AASHTO H-20
24" (600 mm)	2824AG	PEDESTRIAN AASHTO H-10	STANDARD AASHTO H-20	SOLID AASHTO H-20
30" (750 mm)	2830AG	PEDESTRIAN AASHTO H-20	STANDARD AASHTO H-20	SOLID AASHTO H-20

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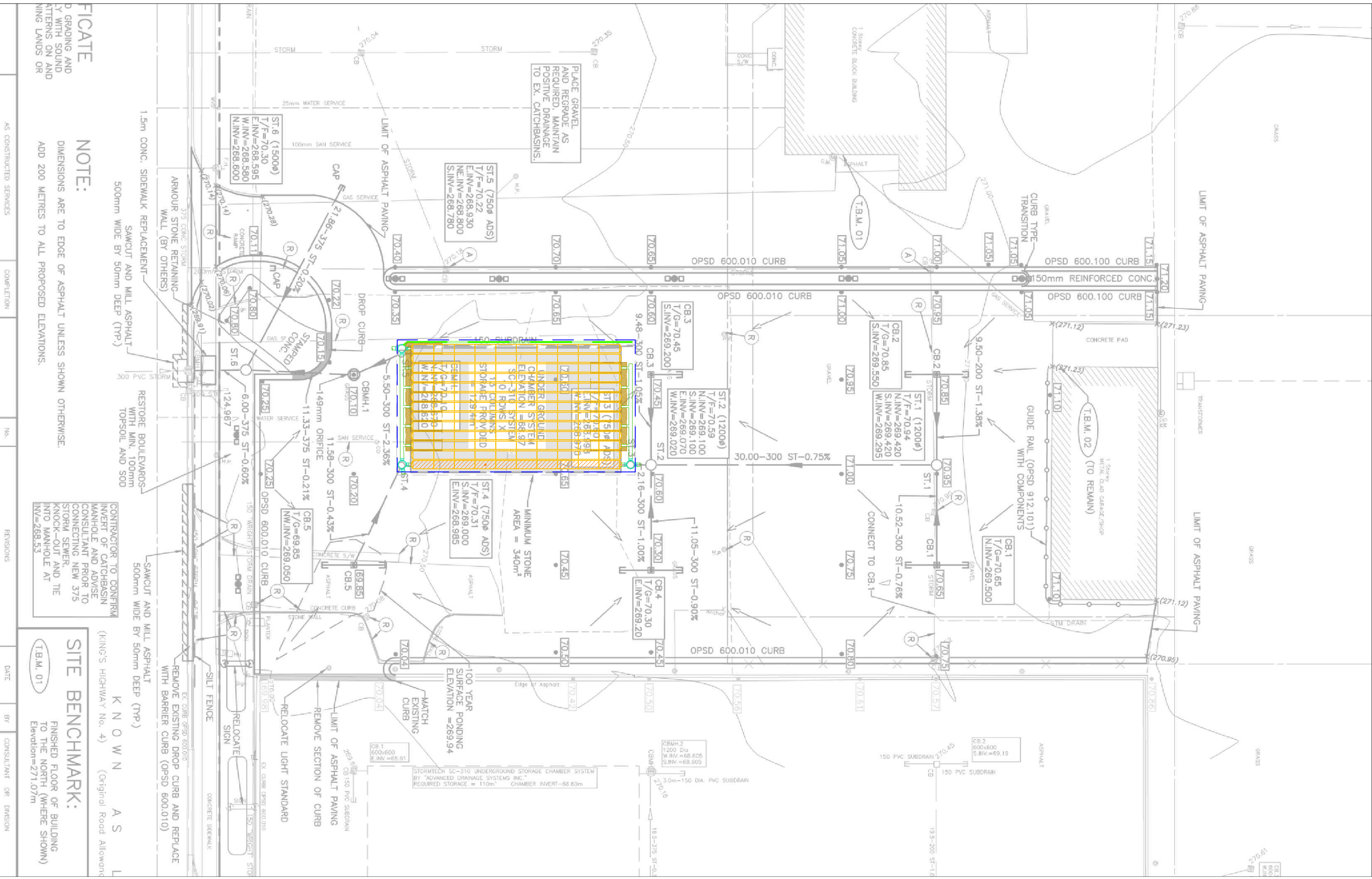


FIGURE:
 D GRADING AND
 Y WITH SOUND
 ATTURNS ON AND
 NING LANDS OR

NOTE:
 DIMENSIONS ARE TO EDGE OF ASPHALT UNLESS SHOWN OTHERWISE.
 ADD 200 METRES TO ALL PROPOSED ELEVATIONS.

CONTRACTOR TO CONFIRM
 INVERT OF CATCHBASIN
 MANHOLE AND ADVISE
 CONSULTANT PRIOR TO
 CONNECTING NEW 375
 STORM SEWER.
 KNOCK-OUT AND TIE
 INTO MANHOLE AT
 INV.=268.53

SITE BENCHMARK:
 FINISHED FLOOR OF BUILDING
 TO THE NORTH (WHERE SHOWN)
 Elevation=271.07m

(KING'S HIGHWAY No. 4) (Original Road Allowance)

K N O W N A S L

AS CONSTRUCTED SERVICES	COMPLETION	No.	REVISIONS	DATE	BY	CONSULTANT OR DIVISION